



PERENCANAAN PEMECAH GELOMBANG (*BREAKWATER*) DAN Pengerukan (*DREDGING*) DI TERMINAL KHUSUS TPPI TUBAN, JAWA TIMUR

OLEH:

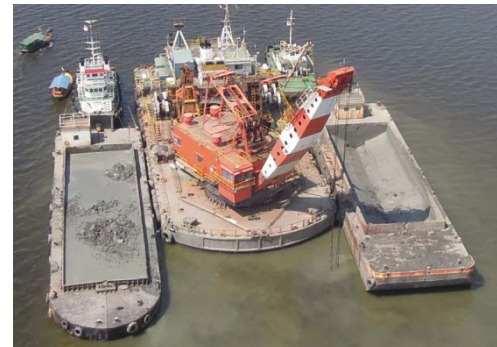
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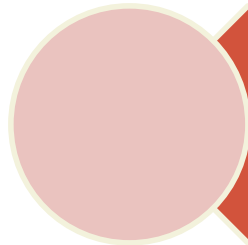
CAHYA BUANA, ST., MT.



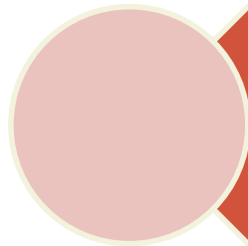


PENDAHULUAN

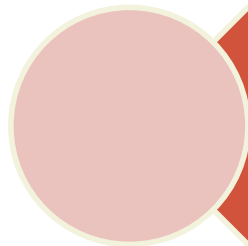
LATAR BELAKANG



Meningkatnya konsumsi minyak di Indonesia



Dibangun Jetty Baru di TPPI Tuban Bagian Utara



Diperlukan Breakwater dan Pengerukan

LATAR BELAKANG



LOKASI

Letak geografis dari kota Tuban adalah $06^{\circ} 45' 38.1''$ LS dan $111^{\circ} 57' 39.0''$ BT.



Rumusan Masalah

- Rumusan masalah dalam perencanaan breakwater ini adalah diperlukannya breakwater baru akibat adanya pengembangan pelabuhan TPPI dibagian utara yang belum terlindung dari gelombang.

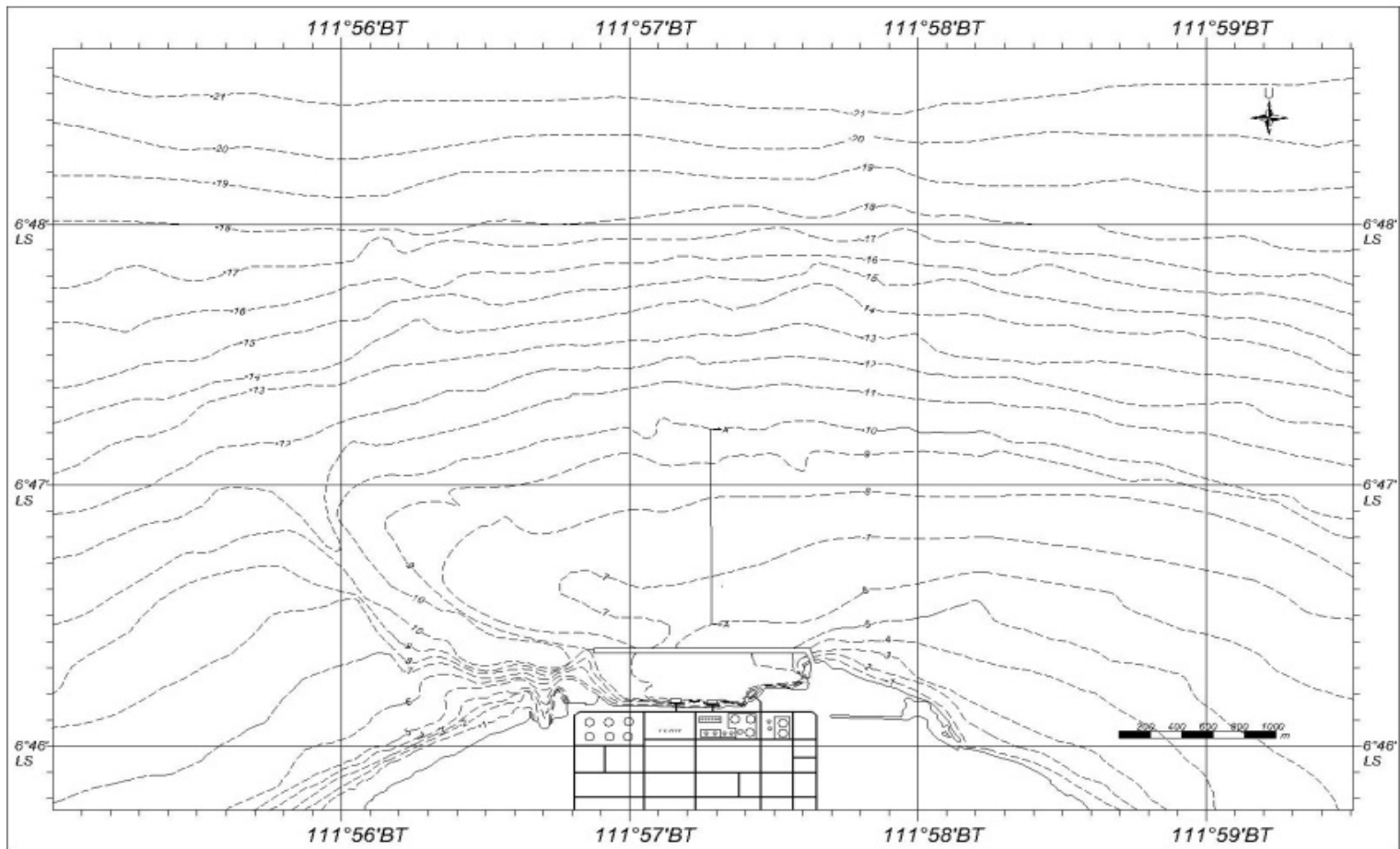


LINGKUP BAHASAN



Data Bathymetri

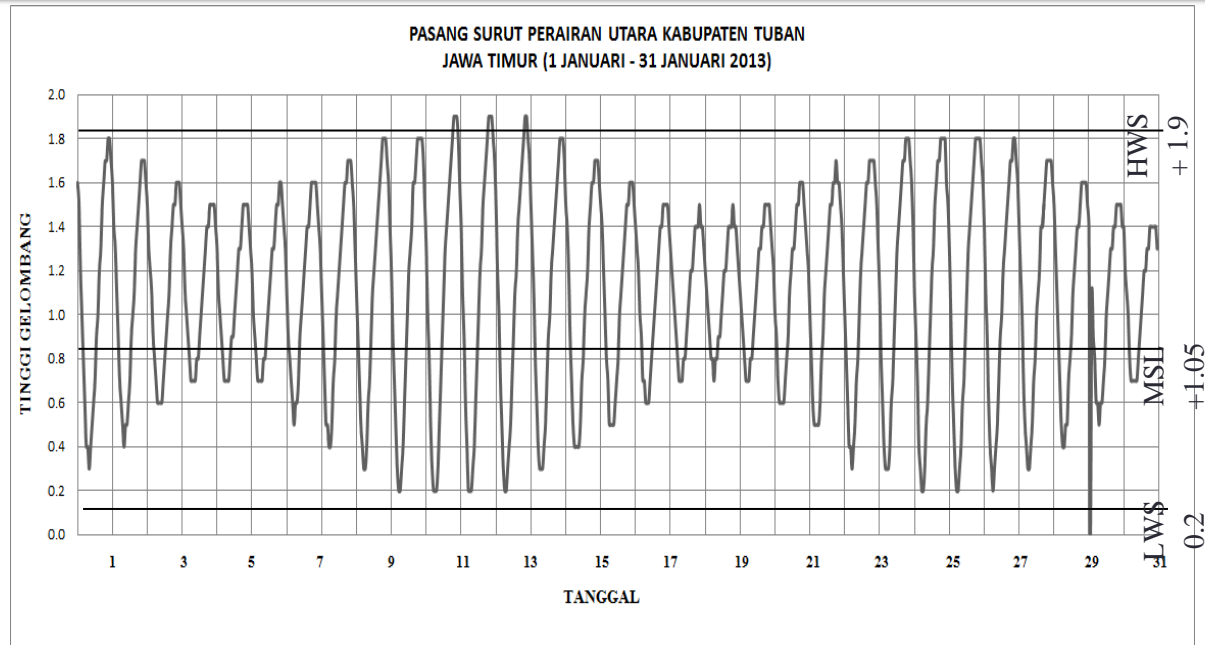
- Kedalaman di lokasi breakwater adalah -7 mLWS
- Kemiringan dasar laut cukup landai 1:300



Data Arus

- Data arus yang digunakan dalam tugas akhir kali ini adalah data yang diperoleh dari keterangan orang lapangan.
- Kecepatan arus yang terjadi cukup rendah sebesar 0.95 m/s.
- arus tidak mengganggu navigasi kapal karena kecepatannya masih di bawah kecepatan ijin 3 knot (1.5 m/s) dan tidak terjadi *cross current*.

Data Pasang Surut



Analisis Data pasang surut sebagai berikut:

- Pasang surut yang terjadi adalah *diurnal* (harian tunggal)
- Beda pasang surut 1.7 m diatas mLWS
- Elevasi HWS (High Water Spring) pada +1.7 mLWS
- Elevasi LWS (Low Water Spring) pada 0.00 mLWS
- Elevasi MSL (Mean Sea Level) pada +1.05 mLWS

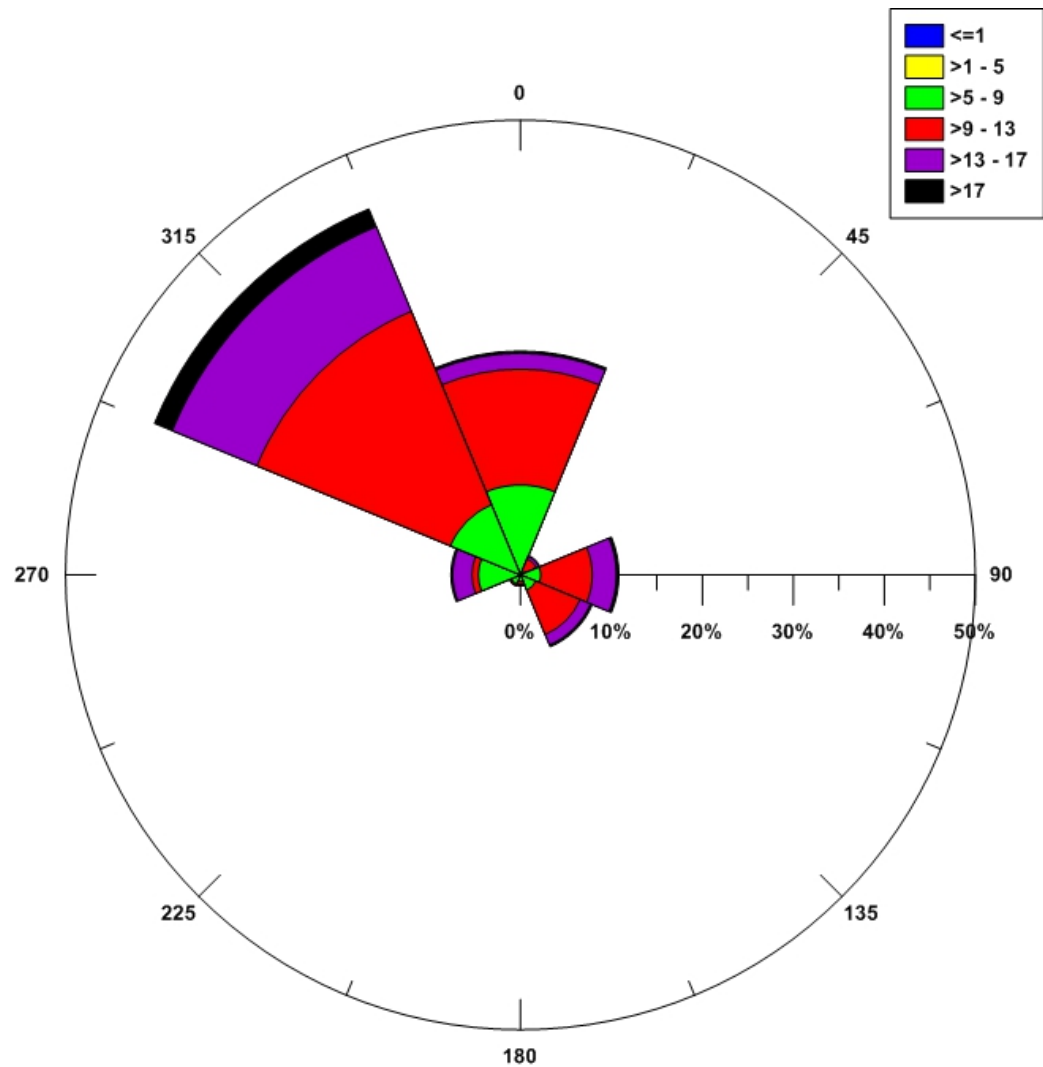
DATA ANGIN

- Data angin didapatkan dari *National Oceanic and Atmospheric Administration (NOAA)* di Stasiun Ahmad Yani, Semarang

Banyaknya Angin yang Terjadi								
KECEPATAN (knot)	ARAH ANGIN							
	utara	timur laut	timur	tenggara	selatan	barat daya	barat	barat laut
1≤	4							
1-5	4	1	4	5	3	1	2	4
5-9	355	18	75	64	24	38	167	300
9-13	455	49	204	188	11	7	23	833
13-17	67	13	95	44	4	1	78	366
≥17	17	3	17	11	3	1	8	85
Jumlah	898	84	395	312	45	48	278	1588
Total	3652							

Persentase Kejadian Angin dalam %									
Kecepatan	Arah Angin								Jumlah
	utara	timur laut	timur	tenggara	selatan	barat daya	barat	barat laut	
1≤	0.11								0.11
1-5	0.11	0.03	0.11	0.14	0.08	0.03	0.05	0.11	0.66
5-9	9.72	0.49	2.05	1.75	0.66	1.04	4.57	8.21	28.50
9-13	12.46	1.34	5.59	5.15	0.30	0.19	0.63	22.81	48.47
13-17	1.83	0.36	2.60	1.20	0.11	0.03	2.14	10.02	18.29
≥17	0.47	0.08	0.47	0.30	0.08	0.03	0.22	2.33	3.97
Jumlah	24.59	2.30	10.82	8.54	1.23	1.31	7.61	43.48	100.00

Windrose



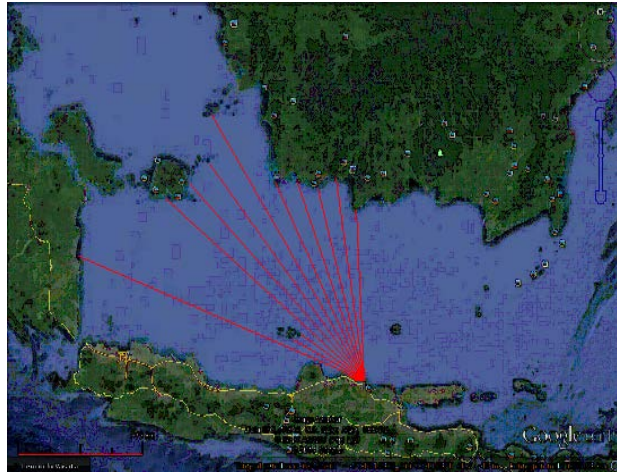


PERAMALAN GELOMBANG

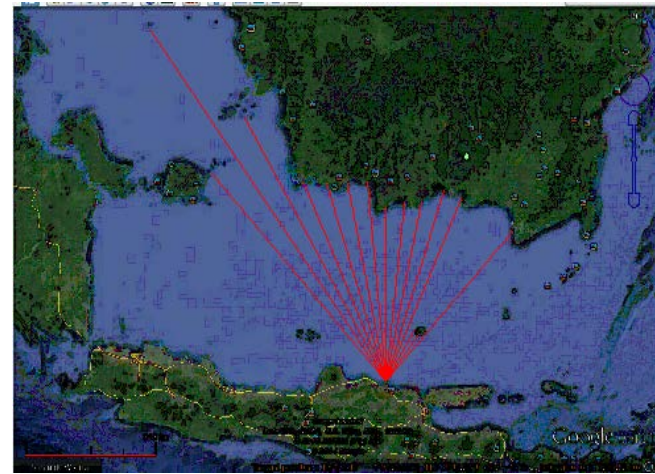
Fetch



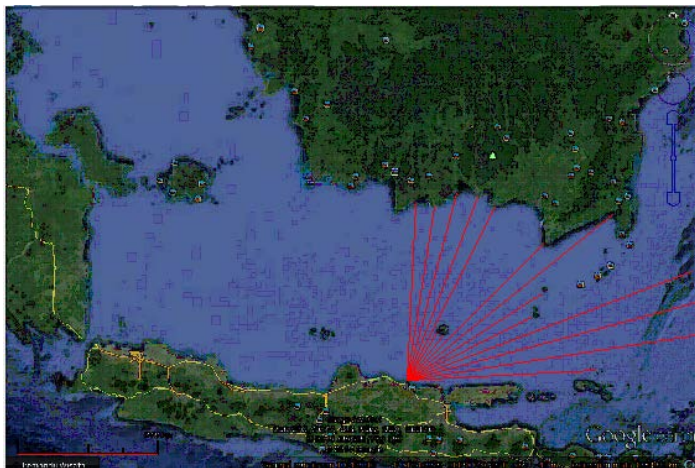
Barat



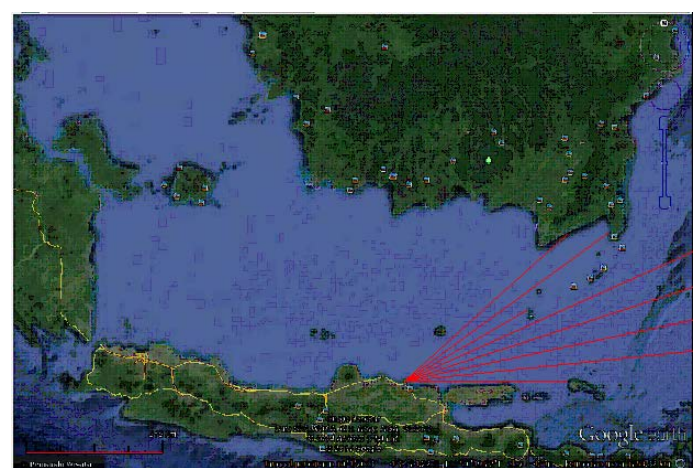
Barat Laut



Utara



Timur Laut



Timur

Fetch Efektif

α	$\cos \alpha$	Xi					Xi.Cos α				
		B	BL	U	TL	T	B	BL	U	TL	T
42	0.743	500.000	386.53	463.43	427.22	0	371.5724	287.25	344.40	317.49	0.00
36	0.809	500.000	465.44	129.05	500	0	404.5085	376.55	104.40	404.51	0.00
30	0.866	198.550	452.41	132.07	500	0	171.9493	391.80	114.38	433.01	0.00
24	0.914	500.000	478.5	456.71	500	0	456.7727	437.13	417.23	456.77	0.00
18	0.951	43.570	500	460.05	319.36	0	41.43753	475.53	437.53	303.73	0.00
12	0.978	35.710	500	422.45	368.4	0	34.92965	489.07	413.22	360.35	0.00
6	0.995	27.560	500	411.88	500	0	27.40902	497.26	409.62	497.26	0.00
0	1.000	25.000	500	398.51	442.41	500	25	500.00	398.51	442.41	500.00
6	0.995	0.000	500	458.9	133.37	500	0	497.26	456.39	132.64	497.26
12	0.978	0.000	192.72	465.85	128.28	500	0	188.51	455.67	125.48	489.07
18	0.951	0.000	205.67	476.05	446.31	500	0	195.60	452.75	424.47	475.53
24	0.914	0.000	500	495.8	449.4	500	0	456.77	452.94	410.55	456.77
30	0.866	0.000	44.2	500	442.75	329.02	0	38.28	433.01	383.43	284.94
36	0.809	0.000	31.51	500	403.28	500	0	25.49	404.51	326.26	404.51
42	0.743	0.000	24.7	500	410.84	500	0	18.36	371.57	305.31	371.57
Total	13.511						1533.58	4874.86	5666.12	5323.67	3479.66
Fetch Efektif (Km)							113.5067	360.8091	419.3736	394.027	257.544



Fetch efektif B = 113,5 km
 Fetch efektif BL = 360,8 km
 Fetch efektif U = 419,4 km
 Fetch efektif TL = 394,0 km
 Fetch efektif T = 257,5 km

Tinggi dan Periode Gelombang

Tahun	Bulan	Kec. Mac		Arah		t	H _{mo}	T _m
		(knot)	(m/s)	(°)	Mata Angin	(jam)	(m)	(s)
2006	Jan	21.0	10.5	330	barat laut	17.57	5.25	11.44
2007	Jul	58.0	29.0	330	barat laut	13.11	12.62	15.33
2008	Apr	40.0	20.0	110	timur	11.91	7.25	12.04
2009	Feb	48.1	24.0	20	utara	15.66	10.80	14.92
2010	Jul	52.9	26.5	110	timur	10.87	9.53	13.19
2011	Jan	22.0	11.0	320	barat laut	17.24	5.55	11.66
2012	Sep	37.9	19.0	100	timur	12.17	6.79	11.78
2013	Jan	36.0	18.0	360	utara	16.84	8.68	13.87
2014	Jun	24.1	12.1	100	barat daya	17.25	5.55	11.65
2015	Mei	38.1	19.1	70	timur	13.76	4.70	10.43

Waktu		Angin Sedang	Angin Kuat
		(11-16 knot)	(17-27 knot)
Musim Barat	Durasi Angin Maksimum	7 jam	6 jam
	Durasi Angin Minimum	5 jam	3 jam
Musim Peralihan	Durasi Angin Maksimum	7 jam	5 jam
	Durasi Angin Minimum	5 jam	2 jam
Musim Timur	Durasi Angin Maksimum	8 jam	4 jam
	Durasi Angin Minimum	6 jam	1 jam

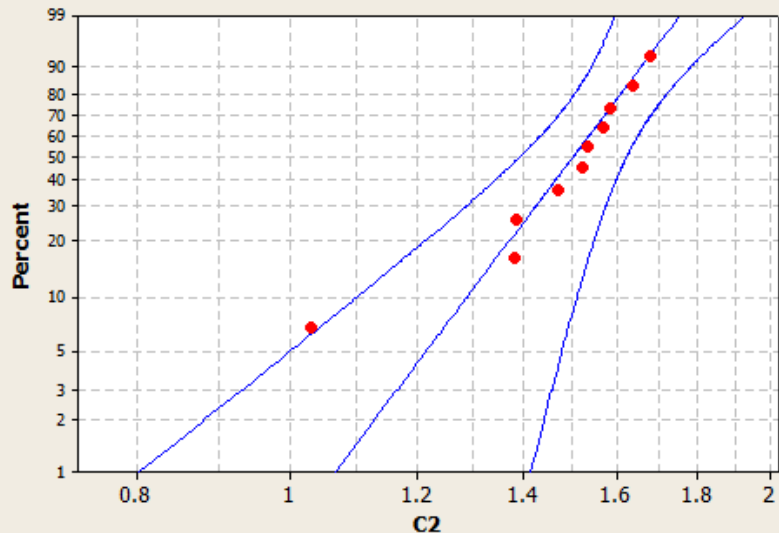
Tinggi dan Periode Gelombang Koreksi

Tahun	Bulan	Kec. Max		Arah		t	H	T
		(Knot)	(m/s)	(°)	Mata Angin	(hr)	(m)	(s)
2006	Jan	21	10.5	330	barat laut	3.5	1.57	5.11
2007	Jul	58.0	29.0	330	barat laut	3	4.56	7.59
2008	Apr	40.0	20.0	110	timur	3	2.58	6.04
2009	Feb	48.1	24.0	20	utara	3	3.42	6.76
2010	Jul	52.9	26.5	110	timur	3	3.96	7.18
2011	Jan	22.0	11.0	320	barat laut	3.5	1.68	5.25
2012	Sep	37.9	19.0	100	timur	3	2.37	5.85
2013	Jan	36.0	18.0	360	utara	3	2.38	5.85
2014	Des	24.1	12.1	100	timur	3	1.50	4.87
2015	Mei	38.1	19.1	70	timur	3	2.39	5.87

PENENTUAN DISTRIBUSI

Probability Plot of Barat

Weibull - 99% CI



Shape	12.31
Scale	1.548
N	10
AD	0.318
P-Value	>0.250

P value = 0,25
= 25%
 α = 1%

P value > α

Berdistribusi Weibull

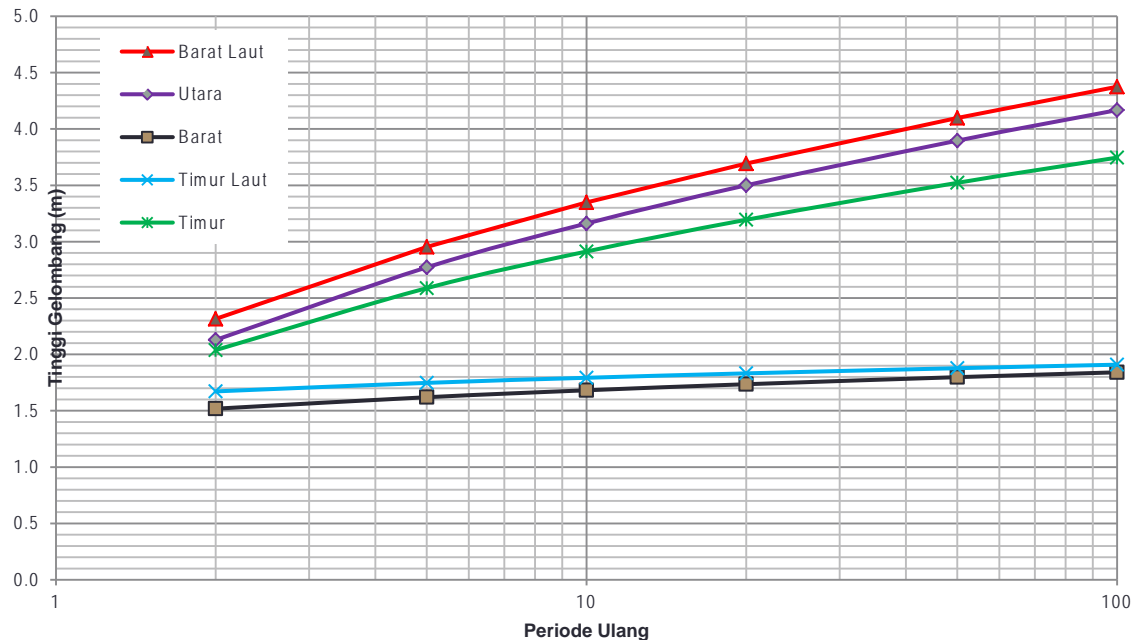
H_0 = Data berdistribusi weibull

H_1 = Data tidak berdistribusi weibull

Jika, P-value < α maka data dinyatakan tolak H_0

Tinggi Gelombang Periode Ulang

Periode Ulang	Tinggi Gelombang Periode Ulang (m)				
(Tahun)	Barat	Barat Laut	Utara	Timur Laut	Timur
2	1.518	2.315	2.128	1.672	2.038
5	1.620	2.954	2.772	1.747	2.588
10	1.682	3.348	3.161	1.792	2.913
20	1.735	3.693	3.500	1.832	3.194
50	1.798	4.098	3.896	1.877	3.522
100	1.841	4.375	4.167	1.909	3.746



Refraksi Gelombang Periode Ulang

utara								
α_0	H	T	d	$K_r = (\cos \alpha_0 / \cos \alpha)^{0.5}$	K_s	$H = K_r * K_s * H$	$\gamma = H/d$	Ket
15	3.90	9.99	10	0.989	0.984	3.79	0.38	Normal
15	3.90	9.99	9	0.988	0.998	3.84	0.43	Normal
15	3.90	9.99	8	0.988	1.019	3.92	0.49	Normal
15	3.90	9.99	7	0.987	1.042	4.01	0.57	Normal

timur								
α_0	H	T	d	$K_r = (\cos \alpha_0 / \cos \alpha)^{0.5}$	K_s	$H = K_r * K_s * H$	$\gamma = H/d$	Ket
85	3.52	9.50	10	0.333	0.969	1.14	0.11	Normal
85	3.52	9.50	9	0.305	0.984	1.06	0.12	Normal
85	3.52	9.50	8	0.324	1.001	1.14	0.14	Normal
85	3.52	9.50	7	0.320	1.023	1.15	0.16	Normal

timur laut								
α_0	H_{s0}	T	d	$K_r = (\cos \alpha_0 / \cos \alpha)^{0.5}$	K_s	$H = K_r * K_s * H$	$\gamma = H/d$	Ket
45	1.88	6.93	10	0.923	0.916	1.59	0.16	Normal
45	1.88	6.93	9	0.915	0.920	1.58	0.18	Normal
45	1.88	6.93	8	0.908	0.928	1.58	0.20	Normal
45	1.88	6.93	7	0.900	0.939	1.59	0.23	Normal

TINGGI GELOMBANG KEDALAMAN
-7 m LWS

Arah	H (m)
Barat	0.588
Barat Laut	3.738
Utara	4.008
Timur Laut	1.587
Timur	1.154

Tinggi Gelombang Berdasarkan Windrose

BARAT DAN UTARA								
Kec. Angin		R _T	R _L	U _w	U _A	t	H _{mo}	T _m
(Knot)	(m/s)					(jam)	(m)	(s)
1-5	1.75	1.1	1.9	3.66	3.50	7	0.36	3.27
5-9	3.75	1.1	1.6	6.60	7.23	7	0.90	4.70
9-13	5.75	1.1	1.5	9.49	11.30	7	1.57	5.87
13-17	7.75	1.1	1.3	11.08	13.68	5	1.55	5.46
≥17	12	1.1	1.05	13.86	18.01	3	1.49	4.85

Timur								
Kec. Angin		R _T	R _L	U _w	U _A	t	H _{mo}	T _m
(Knot)	(m/s)					(jam)	(m)	(s)
1-5	1.75	1.1	1.9	3.66	3.50	8	0.40	3.49
5-9	3.75	1.1	1.6	6.60	7.23	8	0.99	5.02
9-13	5.75	1.1	1.5	9.49	11.30	8	1.73	6.28
13-17	7.75	1.1	1.3	11.08	13.68	6	1.78	5.98
≥17	12	1.1	1.05	13.86	18.01	3	1.49	4.85

Refraksi Berdasarkan Frekuensi Gelombang

UTARA

BARAT LAUT

H	Dari LWS			
(m)	H-10	H-9	H-8	H-7
0.36	0.36	0.36	0.36	0.36
0.90	0.85	0.84		
1.57	1.44	1.43		
1.55	1.41	1.42		
1.49	1.40	1.39		

H	Dari LWS			
(m)	H-10	H-9	H-8	H-7
0.36	0.36	0.36	0.36	0.35
0.90	0.83	0.82	0.80	0.79
1.57	1.36	1.35	1.33	1.33
			1.33	1.32
			1.32	1.29

TIMUR

H	Dari LWS			
(m)	H-10	H-9	H-8	H-7
0.40	0.36	0.32	0.37	0.25
0.99	0.46	0.42	0.40	0.37
1.73	0.63	0.61	0.59	0.58
1.78	0.67	0.64	0.62	0.60
1.49	0.72	0.65	0.62	0.58

BARAT

H	Dari LWS			
(m)	H-10	H-9	H-8	H-7
0.36	0.36	0.31	0.29	0.26
0.90	0.45	0.42	0.39	0.36
1.57	0.60	0.58	0.55	0.54
1.55	0.63	0.64	0.58	0.55
1.49	0.72	0.67	0.62	0.58

H	Dari LWS			
(m)	H-10	H-9	H-8	H-7
0.40	0.40	0.39	0.37	0.38
0.99	0.90	0.89	0.87	0.86
1.73	1.48	1.47	1.47	1.46
1.78	1.53	1.52	1.51	1.50
1.49	1.37	1.34	1.32	1.29

Data Tanah

DEPTH (m)	BORE LOG	Standard Penetration					DESCRIPTION	COLOUR
		Test (SPT) N / 30 cm						
0		0	35	70	105	140	Lime Stone and Clayey Silt	White Brown
1								
2							Lime Stone	White
3								
4								
5								
6								
7								
8								
9								
10								
11								
12								
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								
23								
24								
25								
26								
27								
28								
29								
30								

		Cohesionless Soil / Sol Pulvérulent				
N	(blows)	0 - 3	4 - 10	11 - 30	31 - 50	> 50
γ	(KN/m ³)	-	12 - 16	14 - 18	16 - 20	18 - 23
ϕ	(°)	-	25 - 32	28 - 36	30 - 40	> 35
State		Very Loose	Loose	Medium	Dense	Very Dense
Dr	(%)	0 - 15	15 - 35	35 - 65	65 - 85	85 - 100
		Cohesive Soil / Sol Cohérent				
N	(blows)	< 4	4 - 6	6 - 15	16 - 25	> 25
γ	(KN/m ³)	14 - 18	16 - 18	16 - 18	16 - 20	> 20
q_u	(kPa)	< 25	20 - 50	30 - 60	40 - 200	> 100
Consistency		Very Soft	Soft	Medium	Stiff	Hard

Jenis Tanah = *Limestone*

γ_s = 2 t/m³

ϕ = 36°

Koefisien tekanan tanah aktif:

$$K_a = \tan^2 \left(45 - \frac{36}{2} \right) = 0.259$$

Koefisien tekanan tanah pasif:

$$K_p = \tan^2 \left(45 + \frac{36}{2} \right) = 3.851$$



KRITERIA DESAIN

Material

- **Mutu Beton**

$$\begin{aligned}\sigma'_{bk} &= 30 \text{ Mpa} \\ &= 300 \text{ kg/cm}^2\end{aligned}$$

Modulus Elastisitas berdasarkan PBI 1971

$$E_b = 6400\sqrt{300} \text{ kgf.cm}^{-2} = 1.108 \times 10^5 \text{ kgf.cm}^{-2}$$

- **Mutu Baja Tulangan**

Modulus elastisitas (E_a) : $2.1 \times 10^5 \text{ kgf.cm}^{-2}$

Tegangan leleh karakteristik 3200 kg/cm^2

$$\sigma_a = 1280 \text{ kgf.cm}^{-2}$$

$$\sigma'_{au} = 2780 \text{ kgf.cm}^{-2}$$

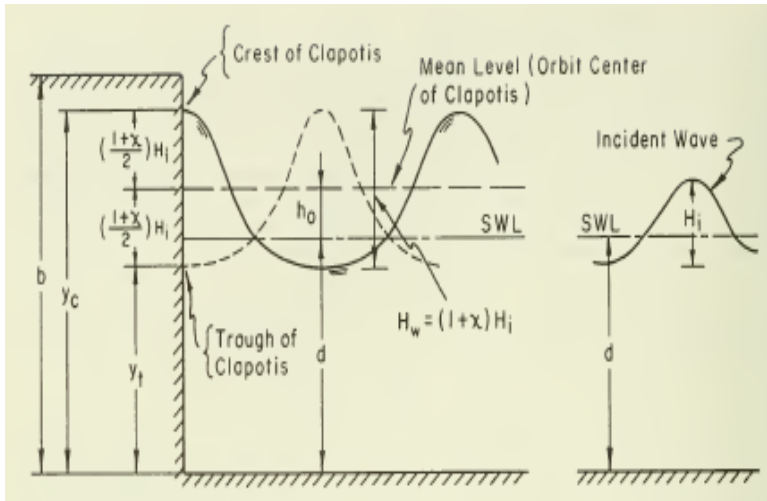
Kriteria Kapal

Spesifikasi Kapal LPG 15000 DWT:

- DWT (Dead Weight Tonage) = 15000 ton
- Displacement Tonage = 20000 ton
- LoA (Panjang kapal) = 146.7 m
- Lpp (Panjang Perpendicular) = 135.5 m
- Lebar kapal (B) = 24 m
- Draft kapal (D) = 9.56 m



Tinggi Struktur



Data perencanaan:

$d = -7.00 \text{ mLWS}$

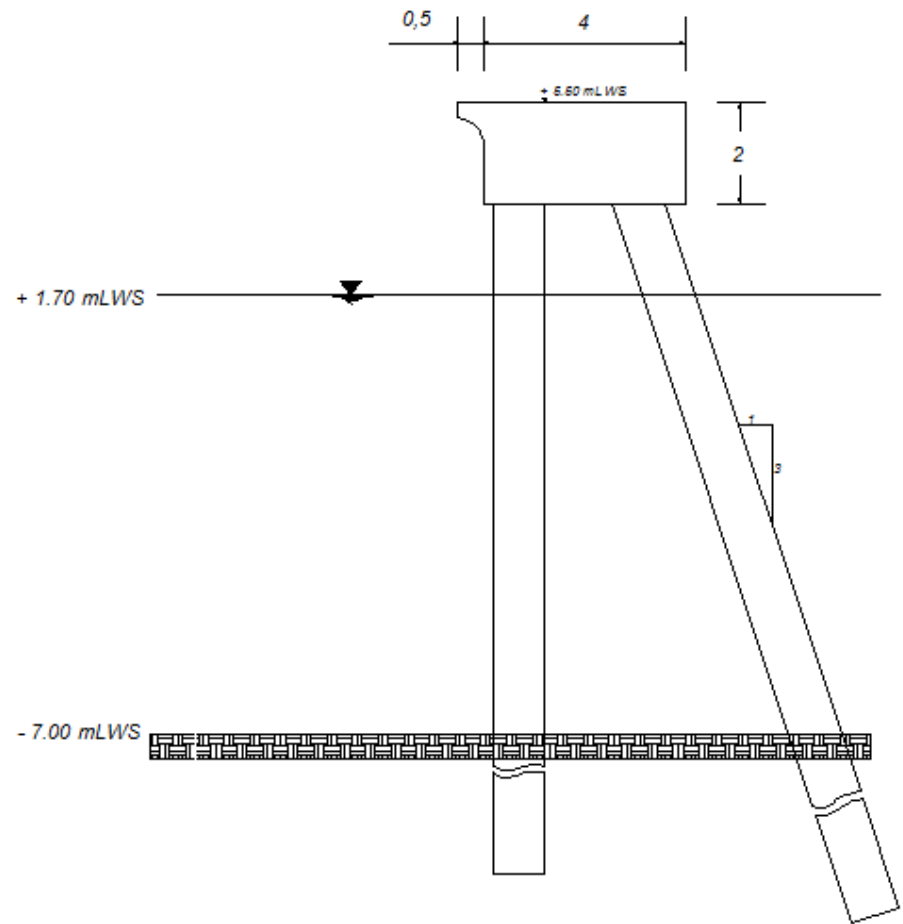
$h_o = 1.70 \text{ m}$

$H_i = 4.06 \text{ m}$

$x = 1$ (dinding tegak)

didapat tinggi breakwater sebesar:

$$\begin{aligned} Y_c &= d + h_o + \left(\frac{1+x}{2}\right) \cdot H_i \\ &= 7 + 1.7 + \left(\frac{1+1}{2}\right) \times 4.06 = 12.76 \text{ m} \\ &= 12.5 \text{ (digunakan)} \end{aligned}$$



Pembebanan (Tekanan Gelombang)

Data gelombang:

$$H = 4.06 \text{ m}$$

$$T = 9.99 \text{ dt}$$

$$\gamma_w = 1.03 \text{ t/m}^3$$

$$H_{\max} = 1.8 \cdot H$$

$$= 1.8 \cdot 4.06 = 7.308 \text{ m}$$

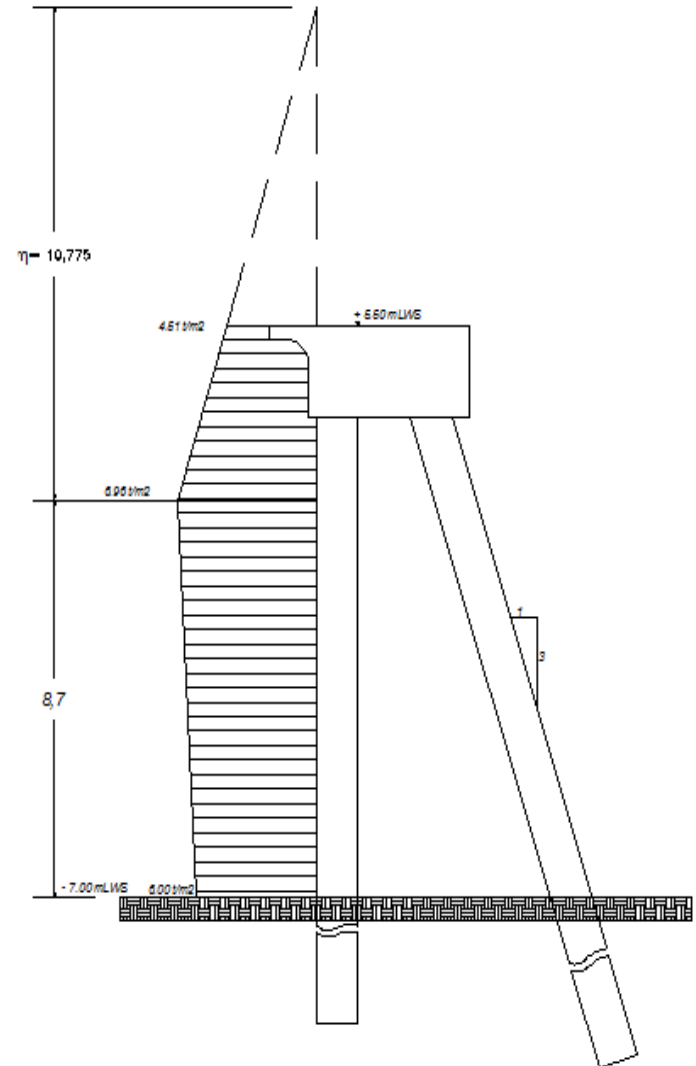
$$\begin{aligned} \eta &= 0.75 (1 + \cos\beta) H_{\max} \\ &= 0.75 (1 + \cos(15^\circ)) \times 7.308 = 10.775 \end{aligned}$$

$$\begin{aligned} P_1 &= \frac{1}{2} (1 + \cos\beta) (\alpha_1 + \alpha_2 \cos^2\beta) \gamma_w \cdot H_{\max} \\ &= 6.96 \text{ t/m}^2 \end{aligned}$$

$$\begin{aligned} P_3 &= \alpha_3 \cdot P_1 \\ &= 6.00 \text{ t/m}^2 \end{aligned}$$

$$\begin{aligned} P_4 &= P_1 (1 - d_c/\eta) \\ &= 4.51 \text{ t/m}^2 \end{aligned}$$

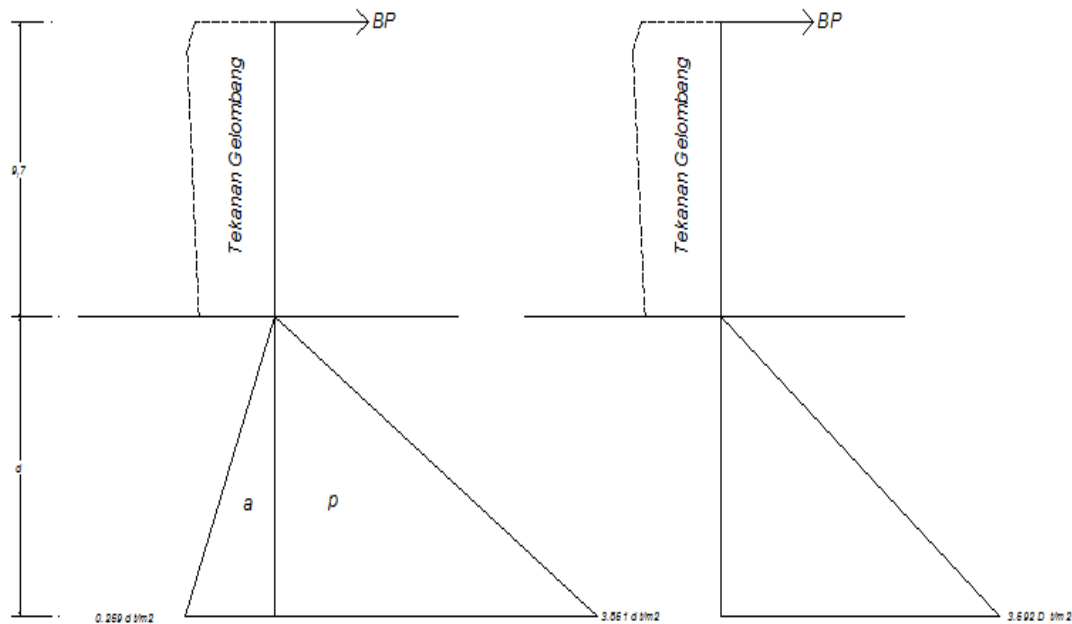
$$\begin{aligned} P &= \frac{1}{2} (P_1 + P_3) h' + \frac{1}{2} (P_1 + P_4) d_c \\ &= 74.346 \text{ ton} \end{aligned}$$



Pembebanan (Tekanan Tanah)

$$\begin{aligned}\sigma_{Ha} &= K_a \times \sigma_v \\ &= 0.259 \times \gamma' \times d \\ &= 0.259 \times (2-1) \times d = 0.259 d\end{aligned}$$

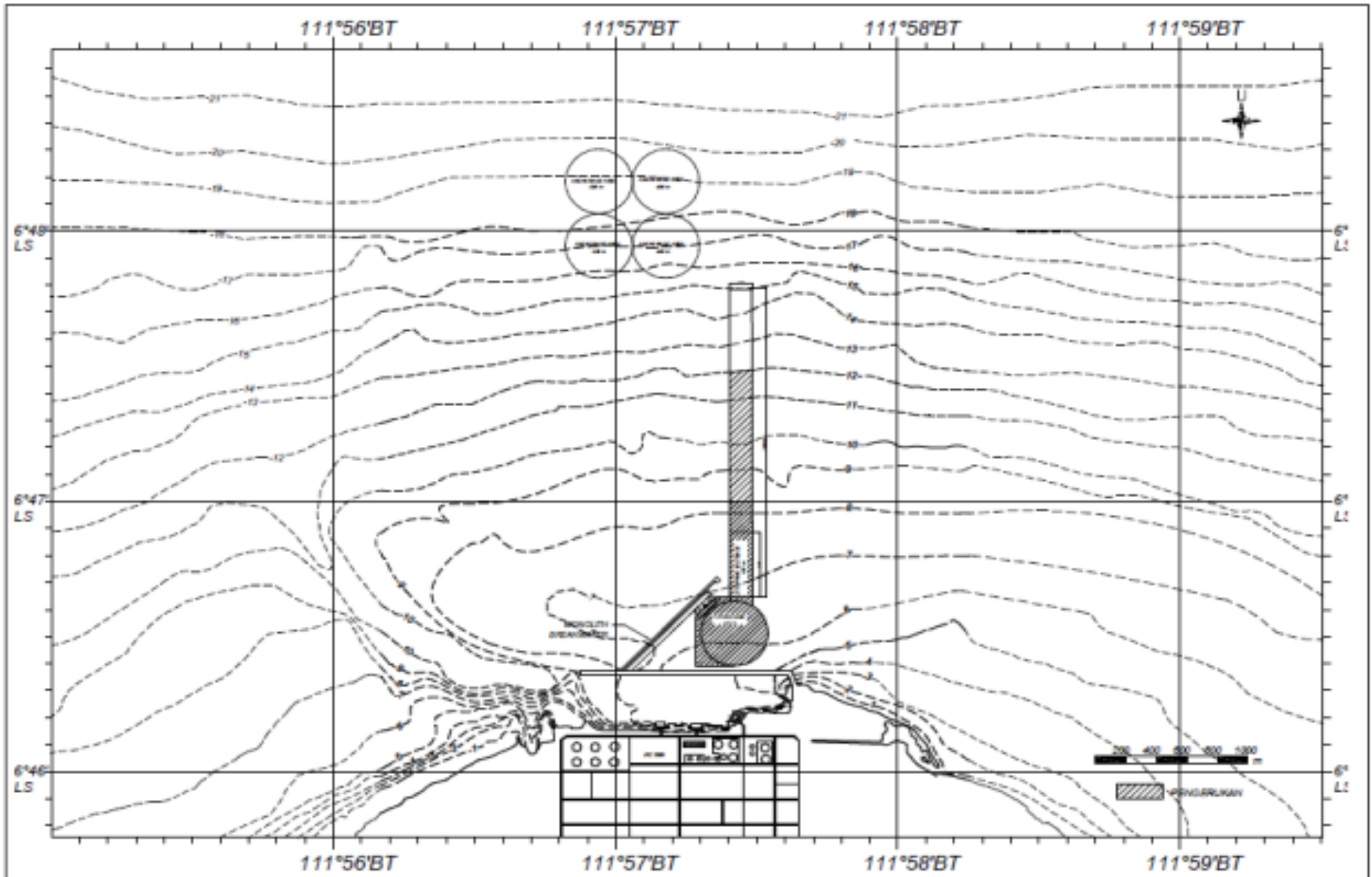
$$\begin{aligned}\sigma_{Hp} &= K_p \times \sigma_v \\ &= 3.851 \times \gamma' \times d \\ &= 3.851 \times (2-1) \times d = 3.851 d\end{aligned}$$





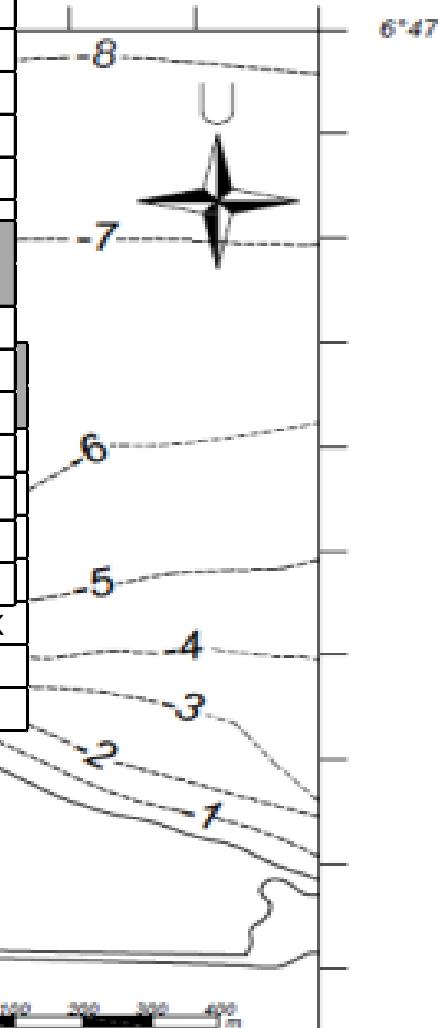
EVALUASI LAYOUT

Perencanaan Layout Perairan



Difraksi Gelombang

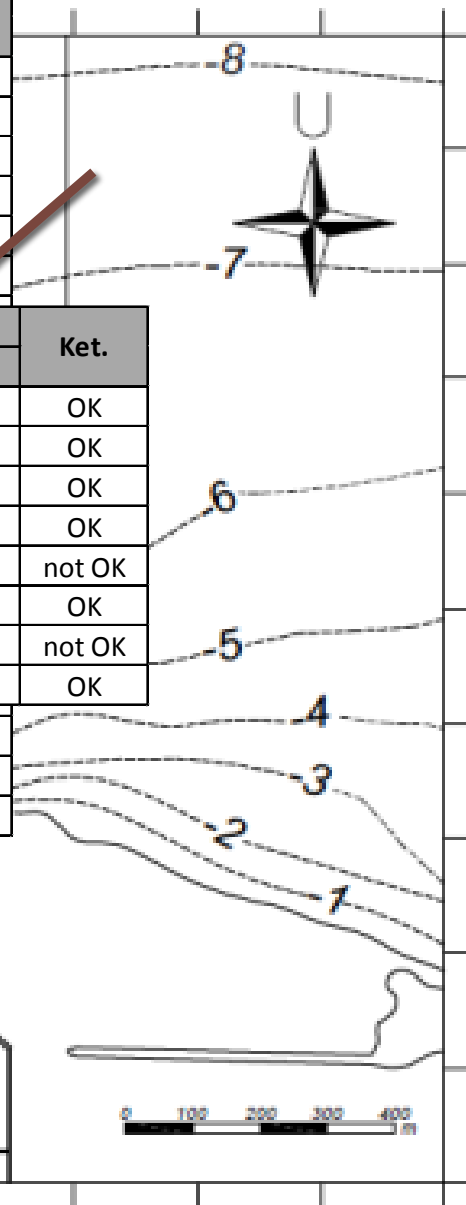
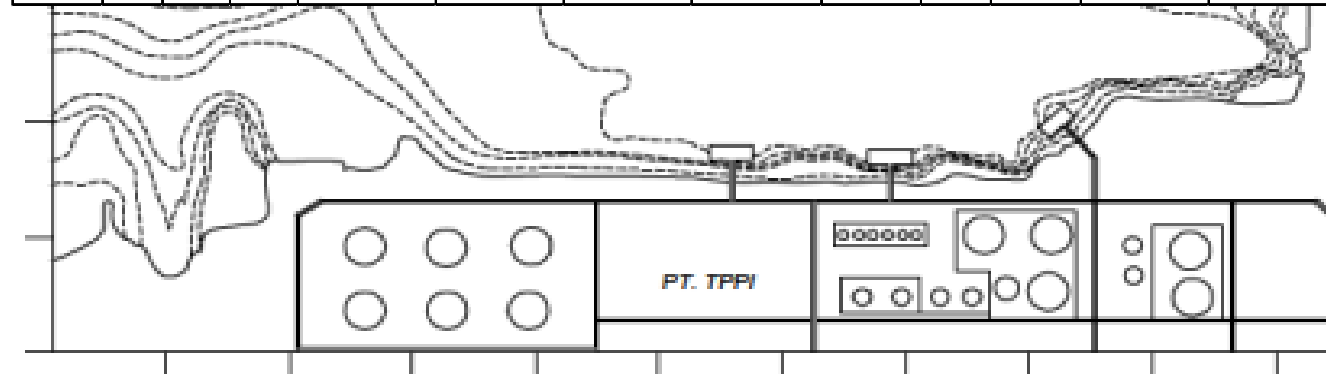
Titik	θ_o (°)	H (m)	T (d)	$L_o = 1,56 T^2$ (m)	X (m)	Y (m)	r (m)	r/L_o	β (°)	K'	H_A (m)	Ket.
1	45	1.44	6.06	57.352	112.33	162.58	197.61	3.45	80	1	1.44	not OK
2	45	1.44	6.06	57.352	55.46	217.37	224.33	3.91	31	0.31	0.45	OK
3	45	1.44	6.06	57.352	108.17	383.08	398.06	6.94	29	0.31	0.45	OK
4	45	1.44	6.06	57.352	112.33	383.08	399.21	6.96	61	0.26	0.37	OK
5	45	1.44	6.06	57.352	332.83	383.08	507.47	8.85	86	1	1.44	not OK
6	45	1.44	6.06	57.352	136.17	603.58	618.75	10.70	32	0.30	0.43	OK
Titik	θ_o (°)	H (m)	T (d)	$L_o = 1,56 T^2$ (m)	X (m)	Y (m)	r (m)	r/L_o	β (°)	K'	H_A (m)	Ket.
1	105	0.58	3.84	23.006	112.33	162.58	197.61	8.59	80	0.17	0.0979	OK
2	105	0.58	3.84	23.006	55.46	217.37	224.33	9.75	31	0.082	0.0472	OK
3	105	0.58	3.84	23.006	108.17	383.08	398.06	17.30	29	0.08	0.0461	OK
4	105	0.58	3.84	23.006	112.33	383.08	399.21	17.35	61	0.09	0.0518	OK
5	105	0.58	3.84	23.006	332.83	383.08	507.47	22.06	86	0.18	0.1037	OK
6	105	0.58	3.84	23.006	136.17	603.58	618.75	26.89	32	0.085	0.0490	OK
7	105	0.58	3.84	23.006	112.33	603.58	613.94	26.69	56	0.085	0.0490	OK
5	75	1.33	5.83	52.941	332.83	383.08	507.47	9.59	86	1	1.33	not OK
6	75	1.33	5.83	52.941	136.17	603.58	618.75	11.69	32	0.11	0.15	OK
7	75	1.33	5.83	52.941	112.33	603.58	613.94	11.60	56	0.17	0.23	OK



PELABUHAN PT. TPPI TANUNG AWAR AWAR

Titik	θ_o (°)	H (m)	T (d)	$L_o = 1,56 T^2$ (m)	X (m)	Y (m)	r (m)	r/ L_o	β (°)	K'	H_A (m)	Ket.
1	90	1.44	6.07	57.477	287.67	163.65	330.96	5.76	29	0.11	0.16	OK
2	90	1.44	6.07	57.477	455.45	218.44	505.12	8.79	25	0.085	0.12	OK
3	90	1.44	6.07	57.477	508.17	384.15	637.03	11.08	37	0.08	0.12	OK
4	90	1.44	6.07	57.477	287.67	384.15	479.92	8.35	53	0.12	0.17	OK
5	90	1.44	6.07	57.477	61.17	384.15	388.99	6.77	80	0.35	0.50	OK
6	90	1.44	6.07	57.477	536.17	604.65	808.13	14.06	48	0.1	0.14	OK
7	90											
8	90											

Titik	θ_o (°)	H (m)	T (d)	$L_o = 1,56 T^2$ (m)	X (m)	Y (m)	r (m)	r/ L_o	β (°)	K'	H_A (m)	Ket.
1	60	1.50	6.19	59.81	287.67	163.65	330.96	5.53	29	0.16	0.24	OK
2	60	1.50	6.19	59.81	455.45	218.44	505.12	8.45	25	0.13	0.19	OK
3	60	1.50	6.19	59.81	508.17	384.15	637.03	10.65	37	0.17	0.25	OK
4	60	1.50	6.19	59.81	287.67	384.15	479.92	8.02	53	0.4	0.60	OK
5	60	1.50	6.19	59.81	61.17	384.15	388.99	6.50	80	1	1.50	not OK
6	60	1.50	6.19	59.81	536.17	604.65	808.13	13.51	48	0.35	0.52	OK
7	60	1.50	6.19	59.81	287.67	604.65	669.59	11.20	64	0.6	0.90	not OK
8	60	1.50	6.19	59.81	300.00	531.60	610.41	10.21	60	0.5	0.75	OK
1	120	1.33	5.83	52.941	536.17	604.65	808.13	15.26	48	0.07	0.09	OK
2	120	1.33	5.83	52.941	287.67	604.65	669.59	12.65	64	0.075	0.10	OK
3	120	1.33	5.83	52.941	300.00	531.6	610.41	11.53	60	0.07	0.09	OK



A thick, orange, hand-painted style arc that starts from the left edge of the frame and curves downwards and to the right, ending near the center of the image.

STRUKTUR BREAKWATER

Perencanaan Sheetpile

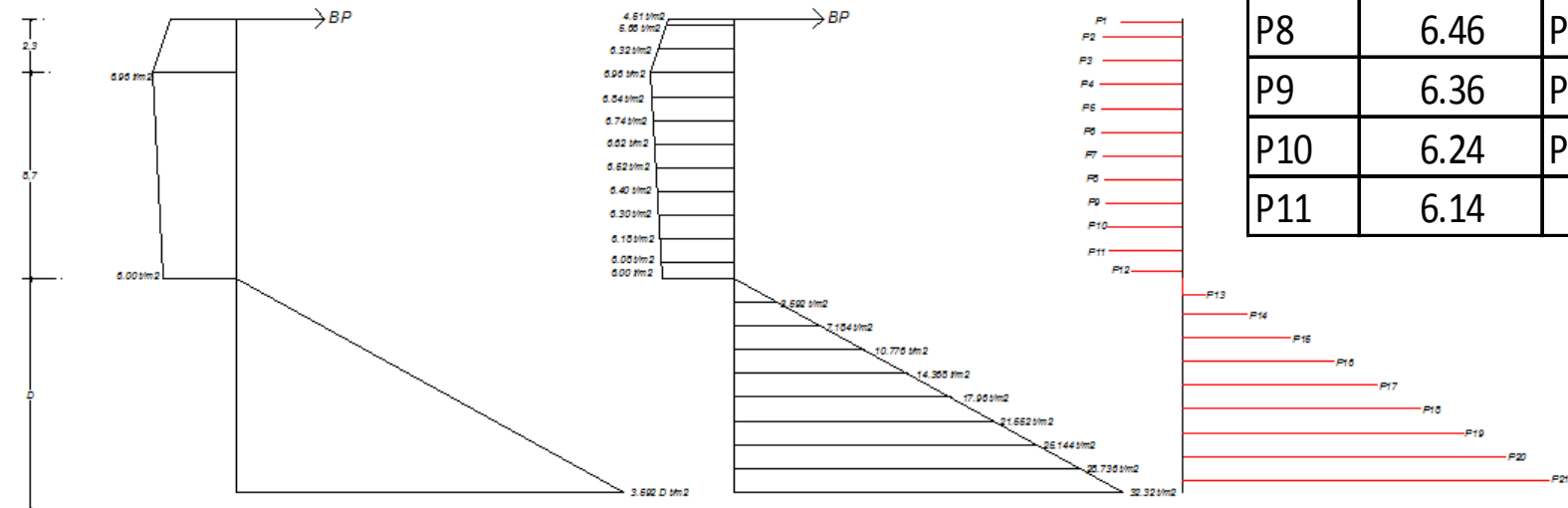
Dalam perhitungan ini dipakai skala sebagai berikut:

Panjang = 1 : 1m
 Lateral = 1 : 2 ton
 Momen = 1 : 20 ton.m

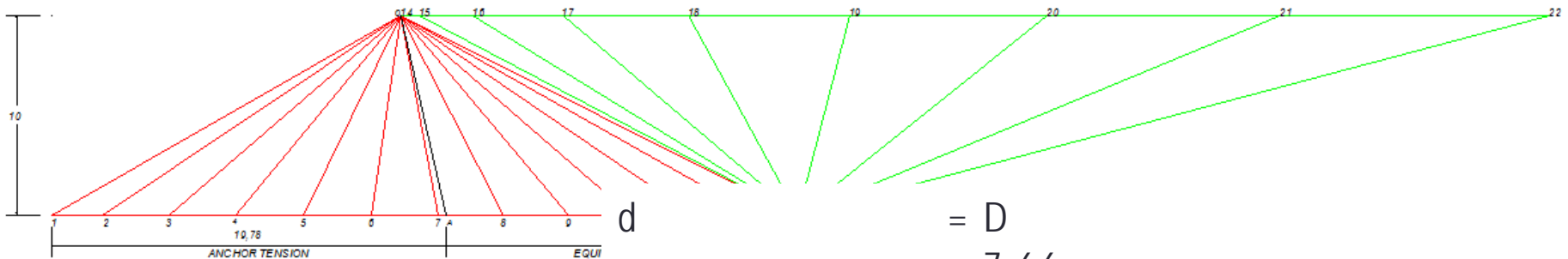
Jarak pole distance :

Pole distane = $20 / (1 \times 2) = 10$

Nama	Gaya (ton)	Nama	Gaya (ton)
P1	5.09	P12	4.22
P2	5.99	P13	1.80
P3	6.64	P14	5.38
P4	6.90	P15	8.98
P5	6.80	P16	12.58
P6	6.68	P17	16.16
P7	6.58	P18	19.76
P8	6.46	P19	23.34
P9	6.36	P20	26.94
P10	6.24	P21	30.80
P11	6.14		



Metode Grafis



Gaya Anchor

= D

= 7.66 m

= Garis A-14 x Skala Lateral

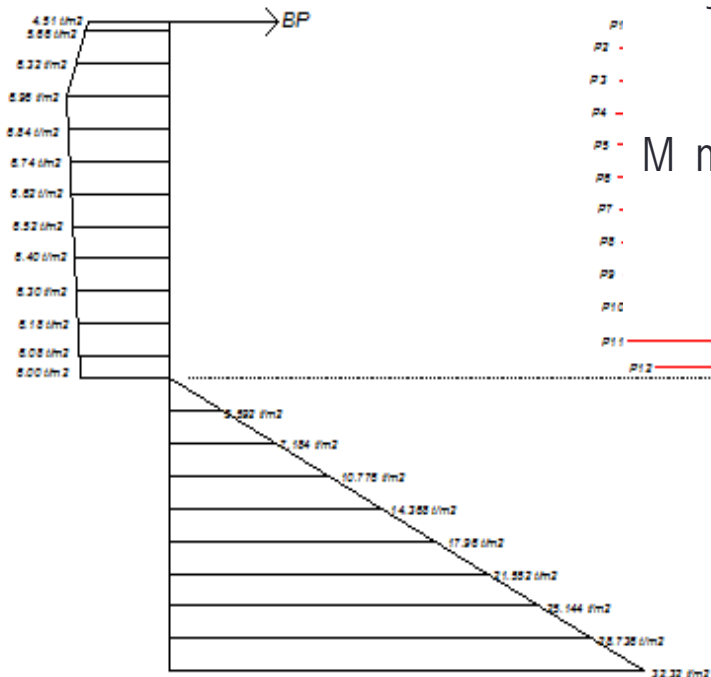
= 19.78 x 2 ton

= 39.56 ton

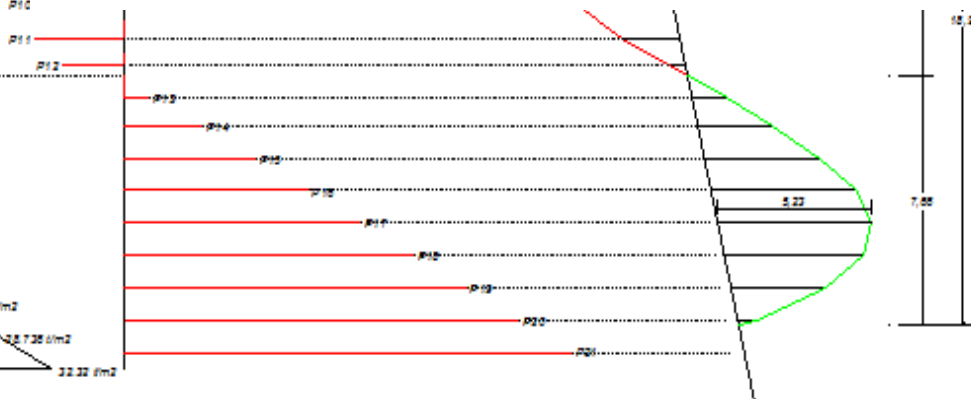
= 5.23 x skala momen

= 5.23 x 20 ton.m

= 104.6 ton.m



M max



Dimensi Sheetpile

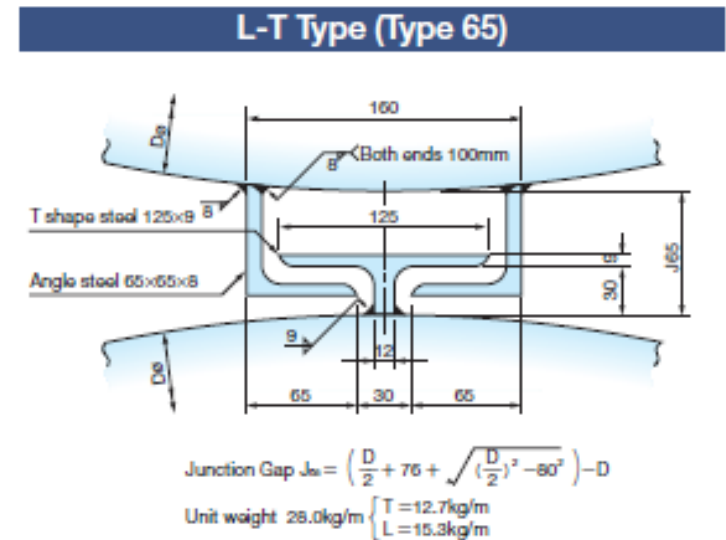
$$Z_0 = \frac{M_{max}}{f_y}$$

$$Z_0 = \frac{104.6}{18200} = 0.00436 \text{ m}^3 = 4360 \text{ cm}^3$$

		Steel Pipe Piles												
Diameter	Thickness	Corrosion allowance (0mm)					Corrosion allowance (1mm)			Per 1m of pile wall width (L – T type) L65 × 65 × 8				
		Sectional area	Unit weight	Moment of inertia	Section modulus	Radius of gyration	Sectional area	Moment of inertia	Section modulus	Junction gap J65 mm	Sectional area	Unit weight	Moment of inertia	Section modulus
mm	mm	cm ²	kg/m	cm ⁴	cm ³	cm	cm ²	cm ⁴	cm ³		cm ² /m	kg/m	cm ⁴	cm ³
500	6	93.1	73.1	284 × 10 ²	114 × 10	17.5	77.4	235 × 10 ²	94.5 × 10	—	—	—	—	—
	7	108.4	85.1	329 × 10 ²	132 × 10	17.4	92.7	281 × 10 ²	113 × 10	—	—	—	—	—
	8	123.7	97.1	374 × 10 ²	150 × 10	17.4	108.0	325 × 10 ²	131 × 10	—	—	—	—	—
	9	138.8	109	418 × 10 ²	167 × 10	17.4	123.2	370 × 10 ²	148 × 10		310.1	243	744 × 10 ²	297 × 10
	10	153.9	121	462 × 10 ²	185 × 10	17.3	138.3	413 × 10 ²	166 × 10		336.9	264	821 × 10 ²	328 × 10
	11	169.0	133	505 × 10 ²	202 × 10	17.3	153.3	457 × 10 ²	183 × 10		363.7	285	898 × 10 ²	359 × 10
	12	184.0	144	548 × 10 ²	219 × 10	17.3	168.3	499 × 10 ²	200 × 10		390.3	306	974 × 10 ²	389 × 10
	13	198.9	156	590 × 10 ²	236 × 10	17.2	183.2	541 × 10 ²	217 × 10		416.8	327	105 × 10 ³	419 × 10
	14	213.8	168	632 × 10 ²	253 × 10	17.2	198.1	583 × 10 ²	234 × 10	62.9	443.2	348	112 × 10 ³	449 × 10
	15	228.6	179	673 × 10 ²	269 × 10	17.2	212.9	624 × 10 ²	251 × 10		469.5	369	120 × 10 ³	478 × 10
16	243.3	191	713 × 10 ²	285 × 10	17.1	227.6	664 × 10 ²	267 × 10		495.7	389	127 × 10 ³	507 × 10	
17	258.0	203	753 × 10 ²	301 × 10	17.1	242.3	704 × 10 ²	283 × 10		521.7	410	134 × 10 ³	535 × 10	
18	272.6	214	793 × 10 ²	317 × 10	17.1	256.9	744 × 10 ²	299 × 10		547.7	430	141 × 10 ³	563 × 10	
19	287.1	225	832 × 10 ²	333 × 10	17.0	271.4	783 × 10 ²	314 × 10		573.5	450	148 × 10 ³	591 × 10	
1000	10	311.0	244	381 × 10 ³	762 × 10	35.0	279.6	342 × 10 ³	685 × 10	—	—	—	—	—
	11	341.8	268	418 × 10 ³	836 × 10	35.0	310.4	379 × 10 ³	759 × 10		352.9	277	391 × 10 ³	781 × 10
	12	372.5	292	455 × 10 ³	909 × 10	34.9	341.1	415 × 10 ³	832 × 10		381.6	300	425 × 10 ³	850 × 10
	13	403.1	316	491 × 10 ³	982 × 10	34.9	371.7	452 × 10 ³	905 × 10		410.3	322	459 × 10 ³	918 × 10
	14	433.7	340	527 × 10 ³	105 × 10 ²	34.9	402.3	488 × 10 ³	978 × 10		438.9	344	493 × 10 ³	986 × 10
	15	464.2	364	563 × 10 ³	113 × 10 ²	34.8	432.8	524 × 10 ³	105 × 10 ²		467.4	367	526 × 10 ³	105 × 10 ²
	16	494.6	388	599 × 10 ³	120 × 10 ²	34.8	463.2	560 × 10 ³	112 × 10 ²		495.8	389	560 × 10 ³	112 × 10 ²
	17	525.0	412	634 × 10 ³	127 × 10 ²	34.8	493.6	595 × 10 ³	119 × 10 ²		524.2	411	593 × 10 ³	119 × 10 ²
	18	555.3	436	670 × 10 ³	134 × 10 ²	34.7	523.9	630 × 10 ³	126 × 10 ²	69.6	552.6	434	626 × 10 ³	125 × 10 ²
	19	585.6	460	705 × 10 ³	141 × 10 ²	34.7	554.2	666 × 10 ³	133 × 10 ²		580.9	456	659 × 10 ³	132 × 10 ²
	20	615.8	483	740 × 10 ³	148 × 10 ²	34.7	584.4	700 × 10 ³	140 × 10 ²		609.1	479	691 × 10 ³	138 × 10 ²
	21	645.9	507	774 × 10 ³	155 × 10 ²	34.6	614.5	735 × 10 ³	147 × 10 ²		637.3	500	724 × 10 ³	145 × 10 ²
	22	675.9	531	809 × 10 ³	162 × 10 ²	34.6	644.6	769 × 10 ³	154 × 10 ²		665.4	522	756 × 10 ³	151 × 10 ²
	23	705.9	554	843 × 10 ³	169 × 10 ²	34.6	674.6	804 × 10 ³	161 × 10 ²		693.4	544	788 × 10 ³	158 × 10 ²
	24	735.9	578	877 × 10 ³	175 × 10 ²	34.5	704.5	838 × 10 ³	168 × 10 ²		721.4	566	820 × 10 ³	164 × 10 ²
	25	765.8	601	911 × 10 ³	182 × 10 ²	34.5	734.4	871 × 10 ³	175 × 10 ²		749.4	588	851 × 10 ³	170 × 10 ²

Dimensi Sheetpile

- Diameter SSPP = 1000 mm
- Tebal = 16 mm
- Junction = 69,6 mm
- Momen Inersia = 560000 cm⁴
- Section Modulus = 11200 cm³
- Fy = 182 Mpa
- Fu = 455 Mpa



Dimensi dan Kedalaman Bearing Pile (Anchor)

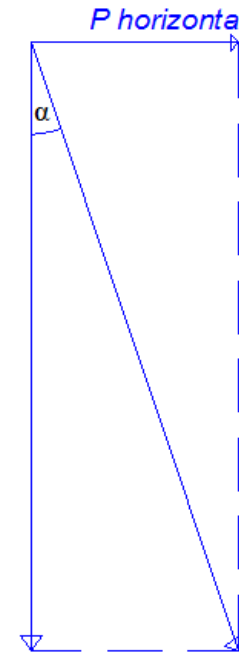
- Dimensi Bearing Pile yang dipakai
- Diameter = 1000 mm
- Tebal = 16 mm
- Mutu = ASTM A252 Grade 3
- F_y = 182 Mpa
- F_u = 455 Mpa

$$\begin{aligned} P_{\text{horizontal}} &= 39.56 \text{ ton/m} \cdot 4,25 \text{ m} \\ &= 168.13 \text{ ton per pile} \end{aligned}$$

$$SF = 3$$

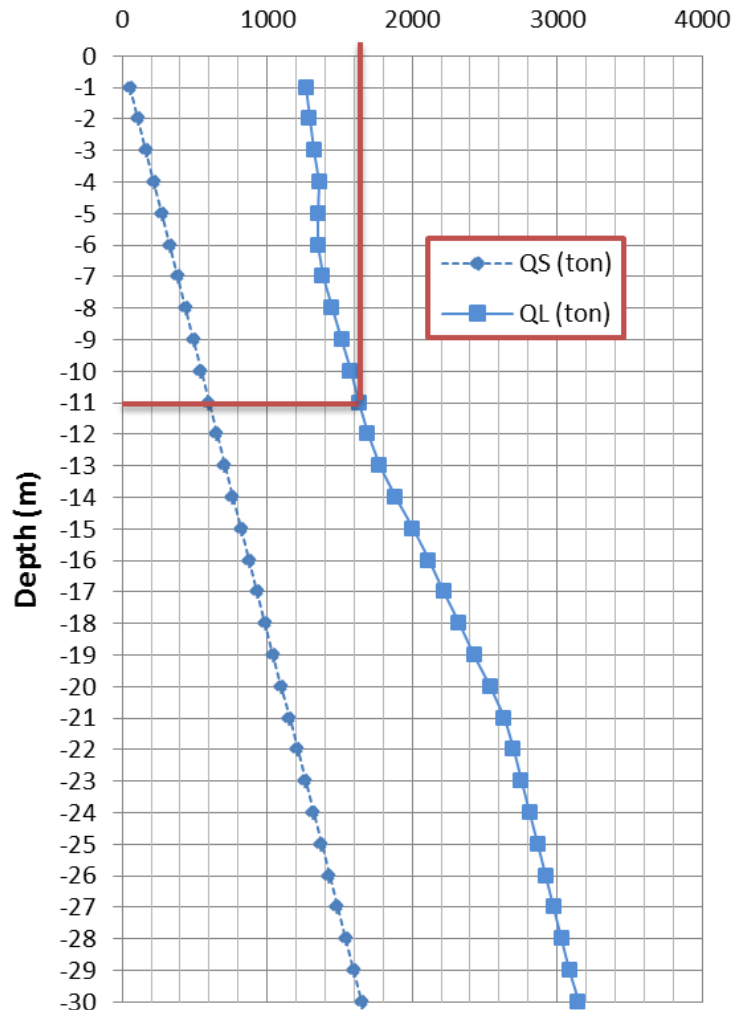
$$Q_{\text{horizontal}} = 504.39$$

$$\begin{aligned} Q_{\text{bearing pile}} &= Q_{\text{horizontal}} / \sin 18.435^\circ \\ &= 504.39 / 0.316 \\ &= 1596.17 \text{ ton} \end{aligned}$$



Daya Dukung Tanah

Daya Dukung Tanah (ton)

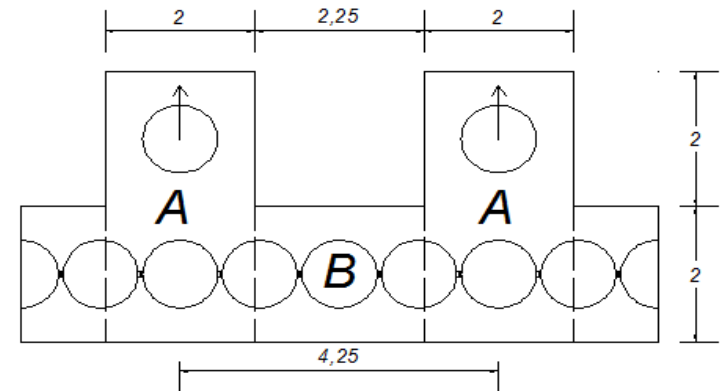


Depth(m)	Elevation(mLWS)	N	N'	Np'	K	Ap	Qp	Ns1	Ns	qs	As	Qs	QL (ton)	Qall (ton)
1.00	-1.000	80	47.5	38.7	40	0.78500	1215.2	50.0	50.0	17.7	3.14	55.5	1270.7	423.6
2.00	-2.000	75	45	37.7	40	0.78500	1182.7	50.0	50.0	17.7	6.28	111.0	1293.7	431.2
3.00	-3.000	60	37.5	36.9	40	0.78500	1159.6	50.0	50.0	17.7	9.42	166.5	1326.1	442.0
4.00	-4.000	48	31.5	36.4	40	0.78500	1142.2	48.0	49.5	17.5	12.57	219.9	1362.1	454.0
5.00	-5.000	49	32	34.4	40	0.78500	1079.4	49.0	49.4	17.5	15.71	274.4	1353.7	451.2
6.00	-6.000	50	32.5	32.6	40	0.78500	1022.5	50.0	49.5	17.5	18.85	329.9	1352.3	450.8
7.00	-7.000	50	32.5	31.8	40	0.78500	998.9	50.0	49.6	17.5	21.99	385.4	1384.3	461.4
8.00	-8.000	50	32.5	32.1	40	0.78500	1006.8	50.0	49.6	17.5	25.13	440.9	1447.6	482.5
9.00	-9.000	48	31.5	32.5	40	0.78500	1020.5	48.0	49.4	17.5	28.27	494.3	1514.8	504.9
10.00	-10.000	46	30.5	32.8	40	0.78500	1030.3	46.0	49.1	17.4	31.42	545.6	1575.9	525.2
11.00	-11.000	48	31.5	33.1	40	0.78500	1038.2	48.0	49.0	17.3	34.56	599.0	1637.2	545.7
12.00	-12.000	52	33.5	33.2	40	0.78500	1042.1	50.0	49.1	17.4	37.70	654.5	1696.6	565.5
13.00	-13.000	56	35.5	33.9	40	0.78500	1065.6	50.0	49.2	17.4	40.84	710.0	1775.6	591.9
14.00	-14.000	55	35	35.8	40	0.78500	1122.6	50.0	49.2	17.4	43.98	765.5	1888.1	629.4
15.00	-15.000	54	34.5	37.8	40	0.78500	1185.4	50.0	49.3	17.4	47.12	821.0	2006.4	668.8
16.00	-16.000	52	33.5	39.5	40	0.78500	1240.3	50.0	49.3	17.4	50.27	876.5	2116.8	705.6
17.00	-17.000	60	37.5	41.0	40	0.78500	1287.4	50.0	49.4	17.5	53.41	932.0	2219.4	739.8
18.00	-18.000	75	45	42.6	40	0.78500	1336.5	50.0	49.4	17.5	56.55	987.5	2324.0	774.7
19.00	-19.000	80	47.5	44.2	40	0.78500	1387.5	50.0	49.4	17.5	59.69	1043.0	2430.5	810.2
20.00	-20.000	80	47.5	45.9	40	0.78500	1442.4	50.0	49.5	17.5	62.83	1098.5	2540.9	847.0
21.00	-21.000	80	47.5	47.2	40	0.78500	1481.7	50.0	49.5	17.5	65.97	1154.0	2635.7	878.6
22.00	-22.000	80	47.5	47.5	40	0.78500	1491.5	50.0	49.5	17.5	69.12	1209.5	2701.0	900.3
23.00	-23.000	80	47.5	47.5	40	0.78500	1491.5	50.0	49.5	17.5	72.26	1265.0	2756.5	918.8
24.00	-24.000	80	47.5	47.5	40	0.78500	1491.5	50.0	49.5	17.5	75.40	1320.5	2812.0	937.3
25.00	-25.000	80	47.5	47.5	40	0.78500	1491.5	50.0	49.6	17.5	78.54	1376.0	2867.5	955.8
26.00	-26.000	80	47.5	47.5	40	0.78500	1491.5	50.0	49.6	17.5	81.68	1431.5	2923.0	974.3
27.00	-27.000	80	47.5	47.5	40	0.78500	1491.5	50.0	49.6	17.5	84.82	1487.0	2978.5	992.8
28.00	-28.000	80	47.5	47.5	40	0.78500	1491.5	50.0	49.6	17.5	87.96	1542.5	3034.0	1011.3
29.00	-29.000	80	47.5	47.5	40	0.78500	1491.5	50.0	49.6	17.5	91.11	1598.0	3089.5	1029.8
30.00	-30.000	80	47.5	47.5	40	0.78500	1491.5	50.0	49.6	17.5	94.25	1653.5	3145.0	1048.3

Bulkhead

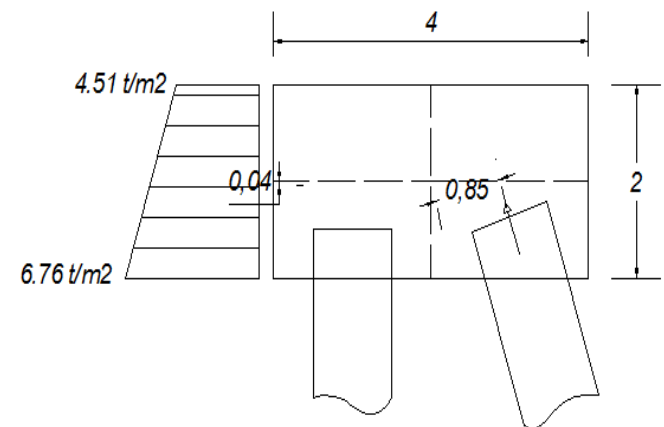
Data data perencanaan

Lebar (b) = 200 cm
 Tinggi (h) = 200 cm
 Panjang = 400 cm
 Selimut beton = 8 cm

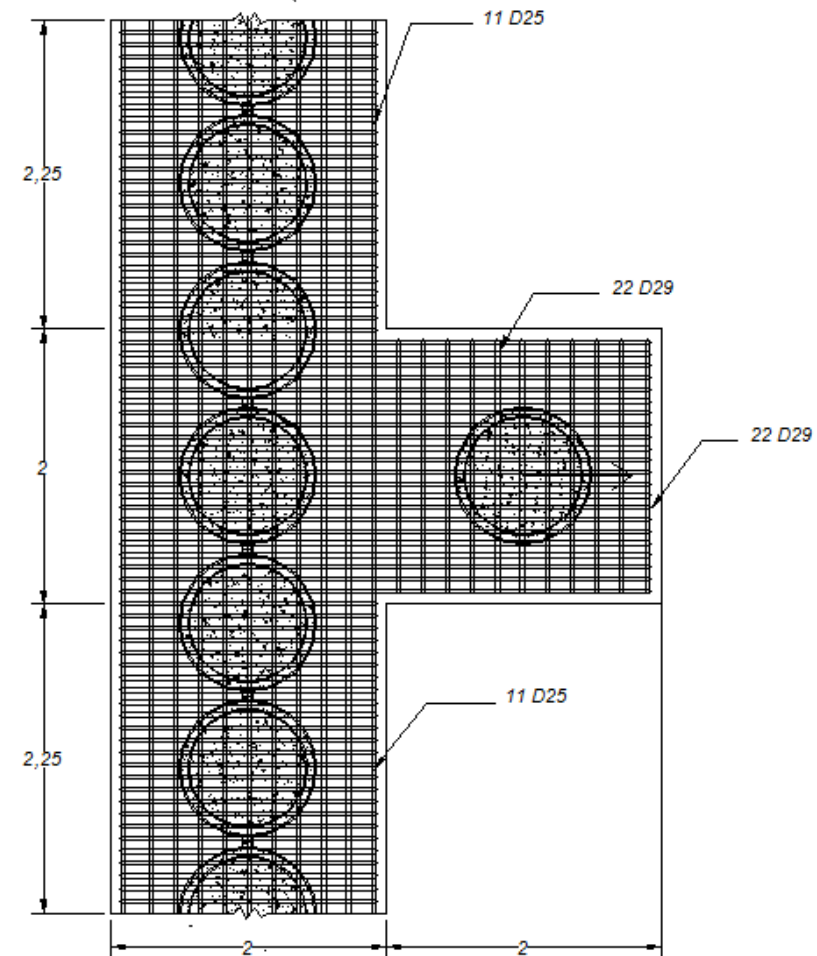
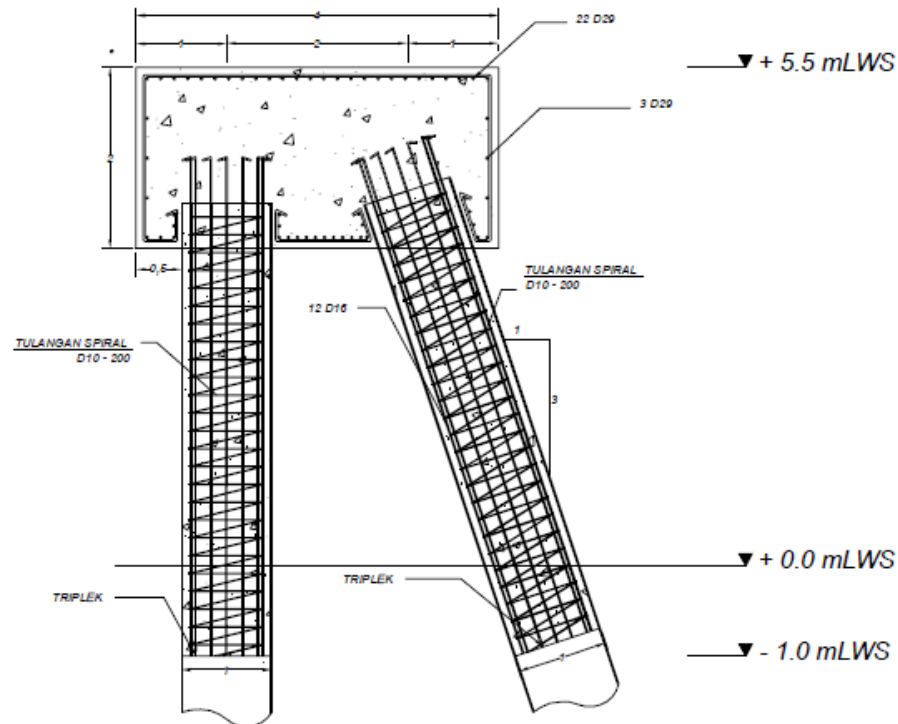


Gaya yang Bekerja

Bearing pile = 168.13 ton/ sin18.435°
 = 531.672
 Tekanan Gelombang = $\frac{(4.51+6.76)}{2} \times 2$
 = 12.24
 Momen = 531.672 x 0.85 + 12.24 x 0.04
 = 452.411 ton.m



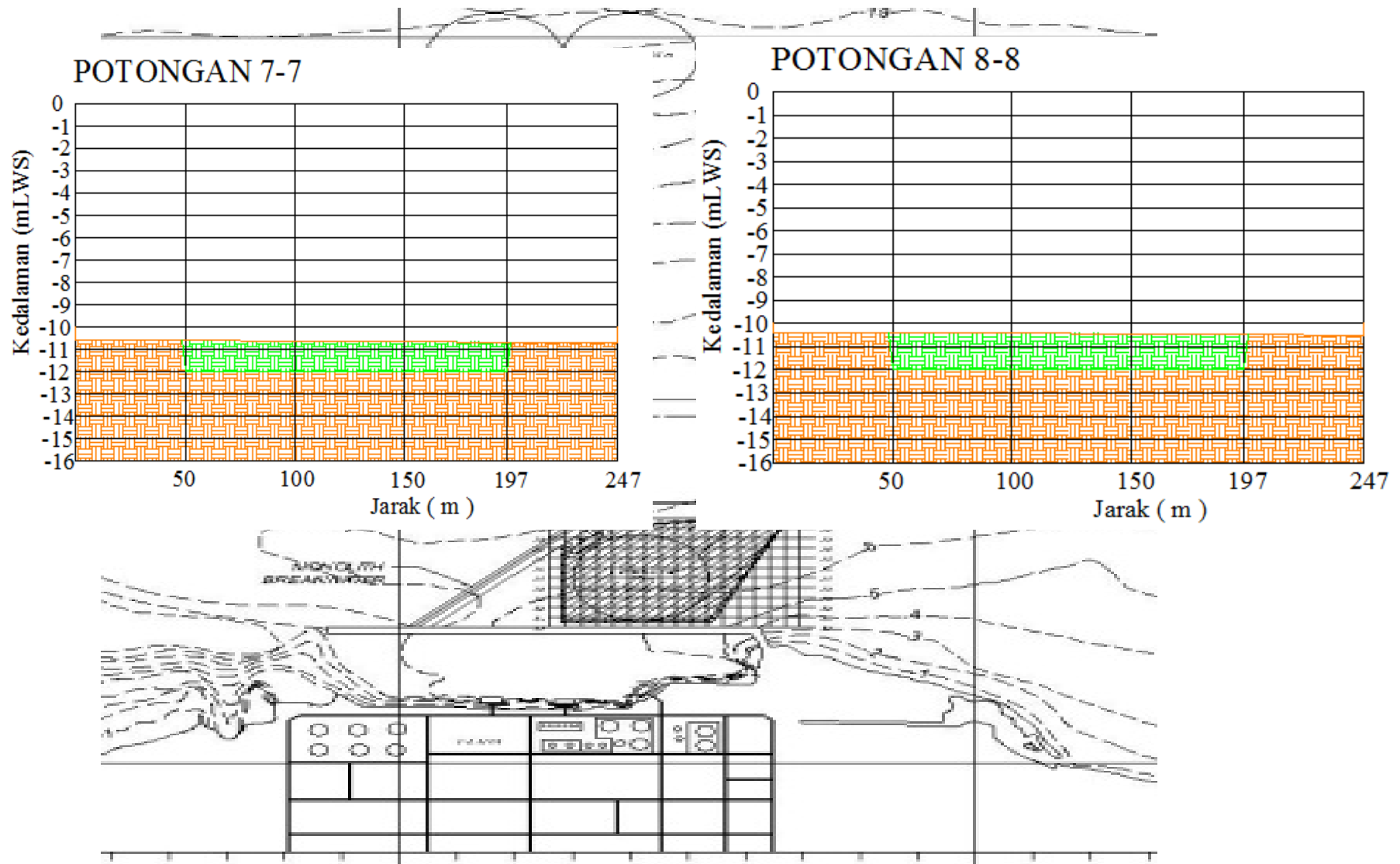
- Tuangan Tarik
22 -D29 dengan luas
- Tulangan Tekan
9 -D29 dengan luas
- Tulangan Samping
3 -D29 dengan luas





PENGERUKAN

Potongan Melintang



Volume Pengerukan

No.	Potongan	A (m ²)	A rata-rata(m ²)	jarak (m)	volume (m ³)
1	1-1	0.00			
2	2-2	30.80	15.40	50.00	770.00
3	3-3	67.22	49.01	50.00	2450.50
4	4-4	103.81	85.52	50.00	4275.75
5	5-5	140.60	122.21	50.00	6110.25
6	6-6	172.51	156.56	50.00	7827.75
7	7-7	203.01	187.76	50.00	9388.00
8	8-8	233.65	218.33	50.00	10916.50
9	9-9	264.13	248.89	50.00	12444.50
10	10-10	295.41	279.77	50.00	13988.50
11	11-11	329.74	312.58	50.00	15628.75
12	12-12	365.17	347.46	50.00	17372.75
13	13-13	400.70	382.94	50.00	19146.75
14	14-14	436.51	418.61	50.00	20930.25
15	15-15	464.89	450.70	50.00	22535.00
16	16-16	491.00	477.95	50.00	23897.25
17	17-17	517.26	504.13	50.00	25206.50
18	18-18	543.59	530.43	50.00	26521.25
19	19-19	569.87	556.73	50.00	27836.50
20	20-20	596.42	583.15	50.00	29157.25
21	21-21	620.27	608.35	50.00	30417.25
22	22-22	643.63	631.95	50.00	31597.50
23	23-23	667.07	655.35	50.00	32767.50
24	24-24	690.54	678.81	50.00	33940.25
25	25-25	715.12	702.83	50.00	35141.50
26	26-26	737.60	726.36	50.00	36318.00
27	27-27	760.66	749.13	50.00	37456.50
28	28-28	777.80	769.23	50.00	38461.50

No.	Potongan	A (m ²)	A rata-rata(m ²)	jarak (m)	volume (m ³)
29	29-29	792.18	784.99	50.00	39249.50
30	30-30	807.22	799.70	50.00	39985.00
31	31-31	897.99	852.61	42.00	35809.41
32	32-32	2992.27	1945.13	50.00	97256.50
33	33-33	3179.20	3085.74	50.00	154286.75
34	34-34	3396.41	3287.81	50.00	164390.25
35	35-35	3485.07	3440.74	50.00	172037.00
36	36-36	3423.32	3454.20	50.00	172709.75
37	37-37	3357.68	3390.50	50.00	169525.00
38	38-38	3290.90	3324.29	50.00	166214.50
39	39-39	3269.90	3280.40	50.00	164020.00
40	40-40	3254.92	3262.41	50.00	163120.50
41	41-41	3235.35	3245.14	50.00	162256.75
42	42-42	0.00	1617.68	50.00	80883.75
VOLUME TOTAL					2354248.66

$$\begin{aligned}
 \text{Volume} &= \text{bulking factor} \times \text{volume pengerukan} \\
 &= 1,4 \times 2.354.248 \\
 &= 3.295.947,2 \text{ m}^3
 \end{aligned}$$

Pemilihan Kapal Keruk

Type of Soil			Type of Dredger				
Klasifikasi	State	N Value	CSD	TSHD	Grab	Bucket	Dipper
Clay	Soft mud	1 - 4					
	Soft	4 - 10					
	Medium	10 - 20					
	Hard	20 - 30					
	Harder	30 - 40					
	Hardest	40 - 50					
Sand	Soft	1 - 10					
	Medium	10 - 20					
	Hard	20 - 30					
	Harder	30 - 40					
	Hardest	40 - 50					
Clay w/ Gravel	Soft	1 - 30					
	Hard	30 -					
Rock	Softer	40 - 50					
	Soft	50 - 60					
Gravel	Medium						
	Hard						
	Hardest						
	Loose						
	Packed						



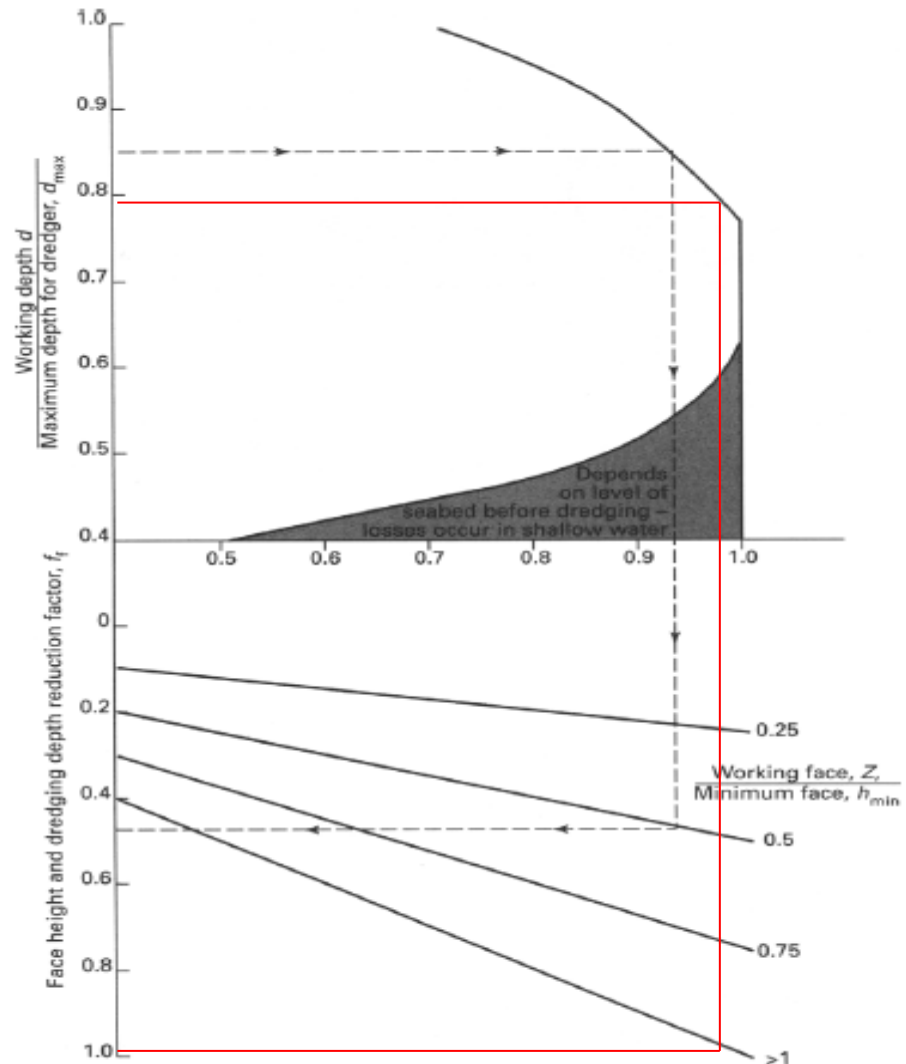
Name		CSD500
Type		Cutter Suction Dredger
Dimension	Length o.a.	19.00 m
	Length over Pontoons	11.50 m
	Beam o.a	4.20 m
	Draught	1.00 m
Dredging Feature	min/max dredging depth	2.5/14 m
	Dredging Width	39.60 m
	Max. Mixture Capacity	4000 m ³ /jam

Produktivitas Alat Keruk

Produktivitas = Kapasitas x kandungan tanah
 = 4000 x 40%
 = 1600 m³/jam

$P_{nom} = f_f \times \text{Produktivitas}$
 = 0.95 x 1600
 = 1520 m³/jam

$P_{max} = f_p \cdot f_a \cdot f_h \cdot P_{nom}$
 = 0.81 x 0.97 x 0.85 x 1520
 = 1015,12 m³/jam

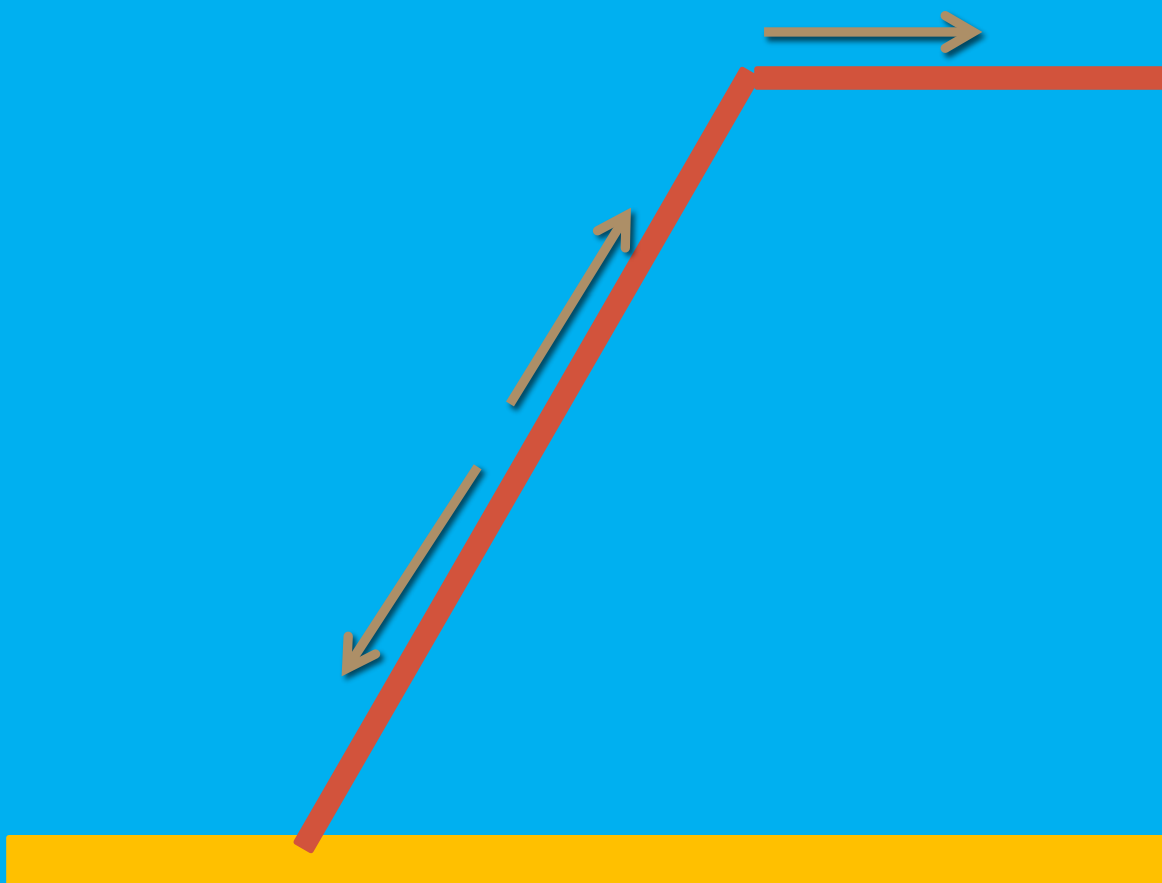


Waktu Pengerukan

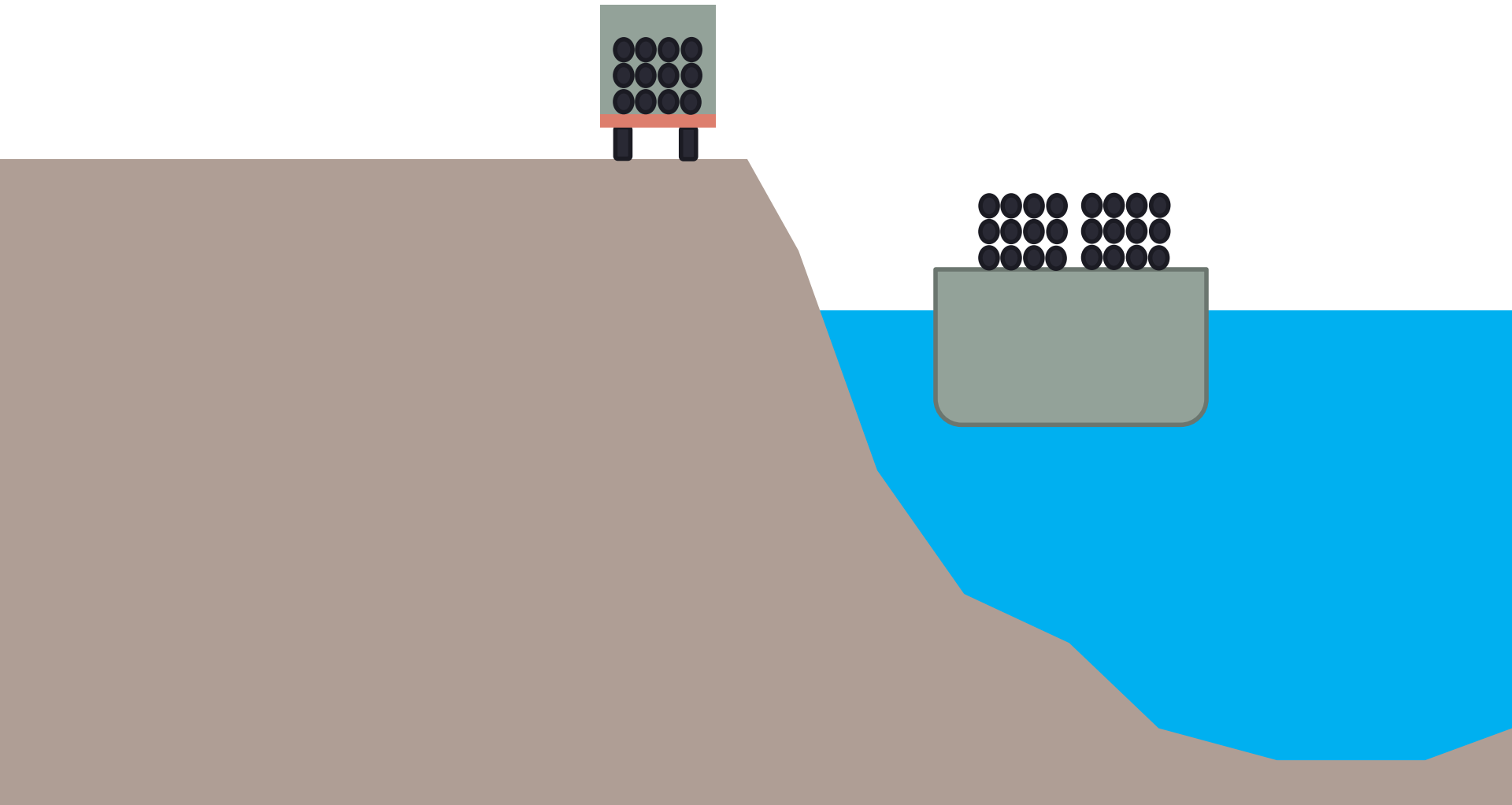
$$\begin{aligned} \cdot T &= \frac{V}{P_{max} \times n} \\ &= \frac{3295947,2}{1015,12 \times 1} = 3246,85 \text{ jam} \\ &= 4,51 \text{ bulan} \end{aligned}$$



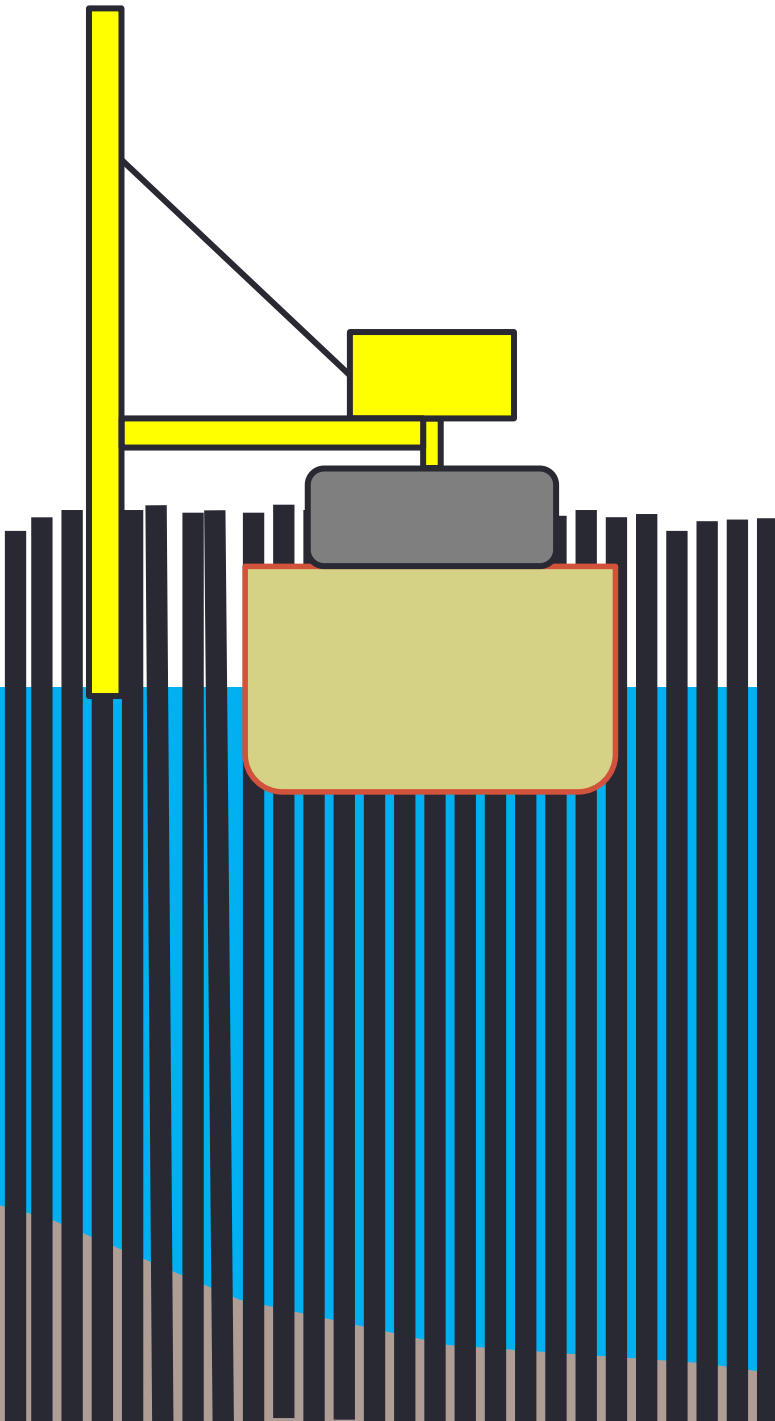
METODE PELAKSANAAN

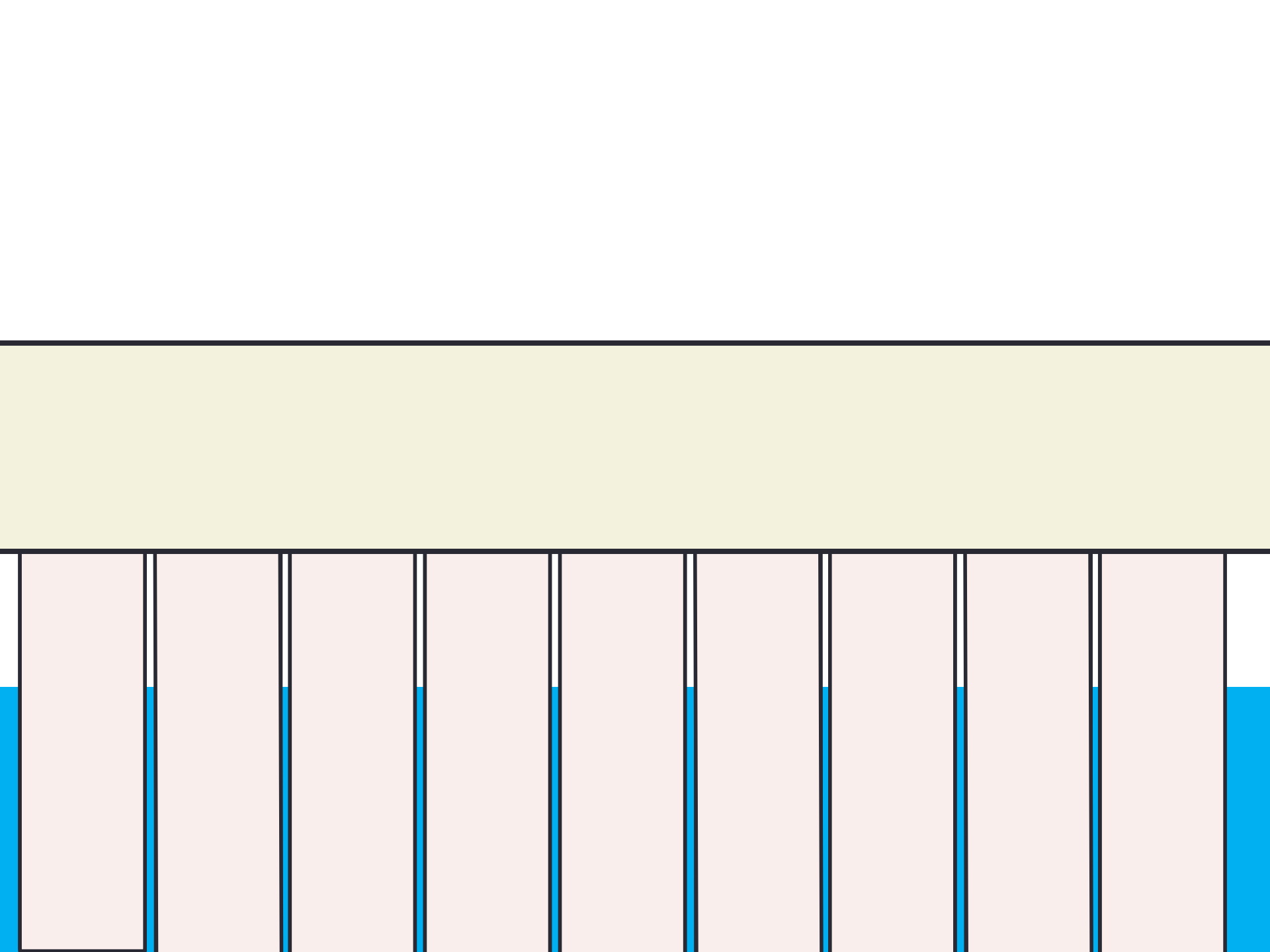


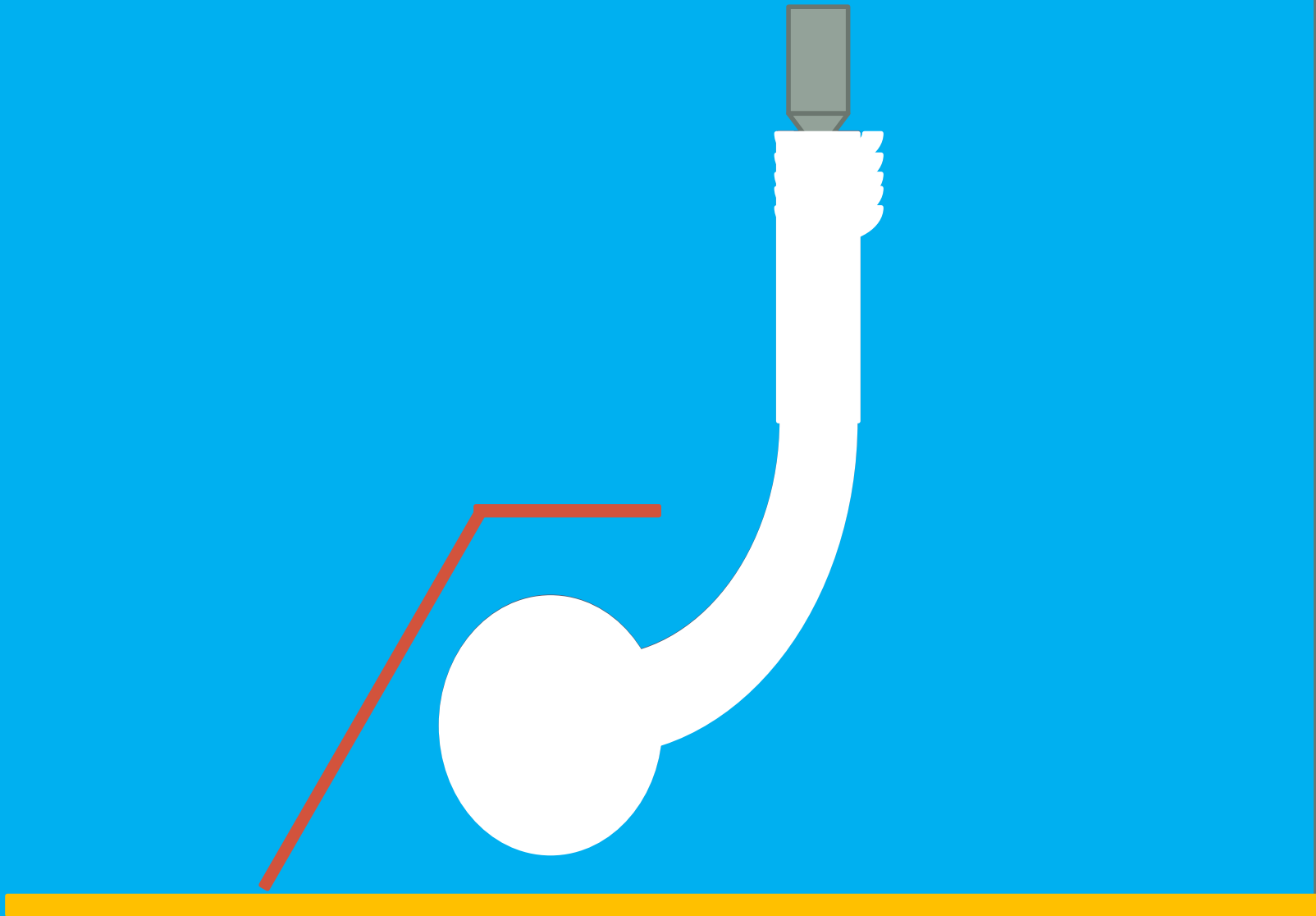
Pengiriman Material



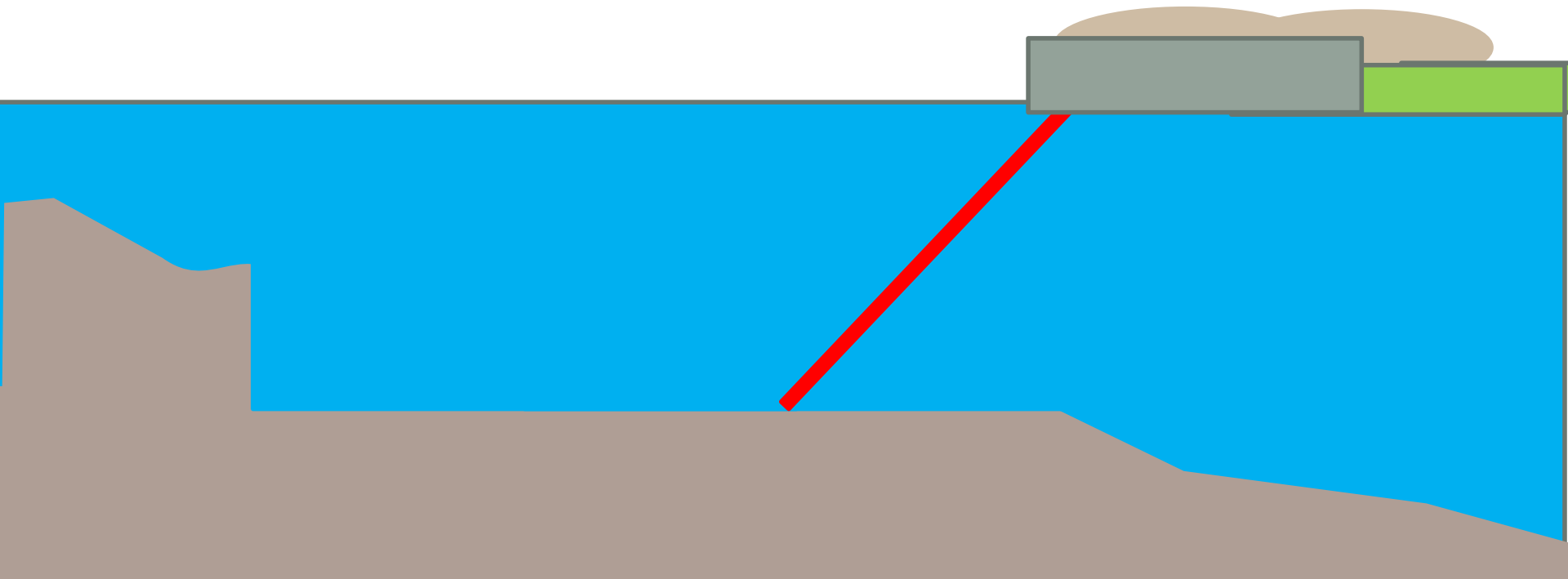
Pemancangan







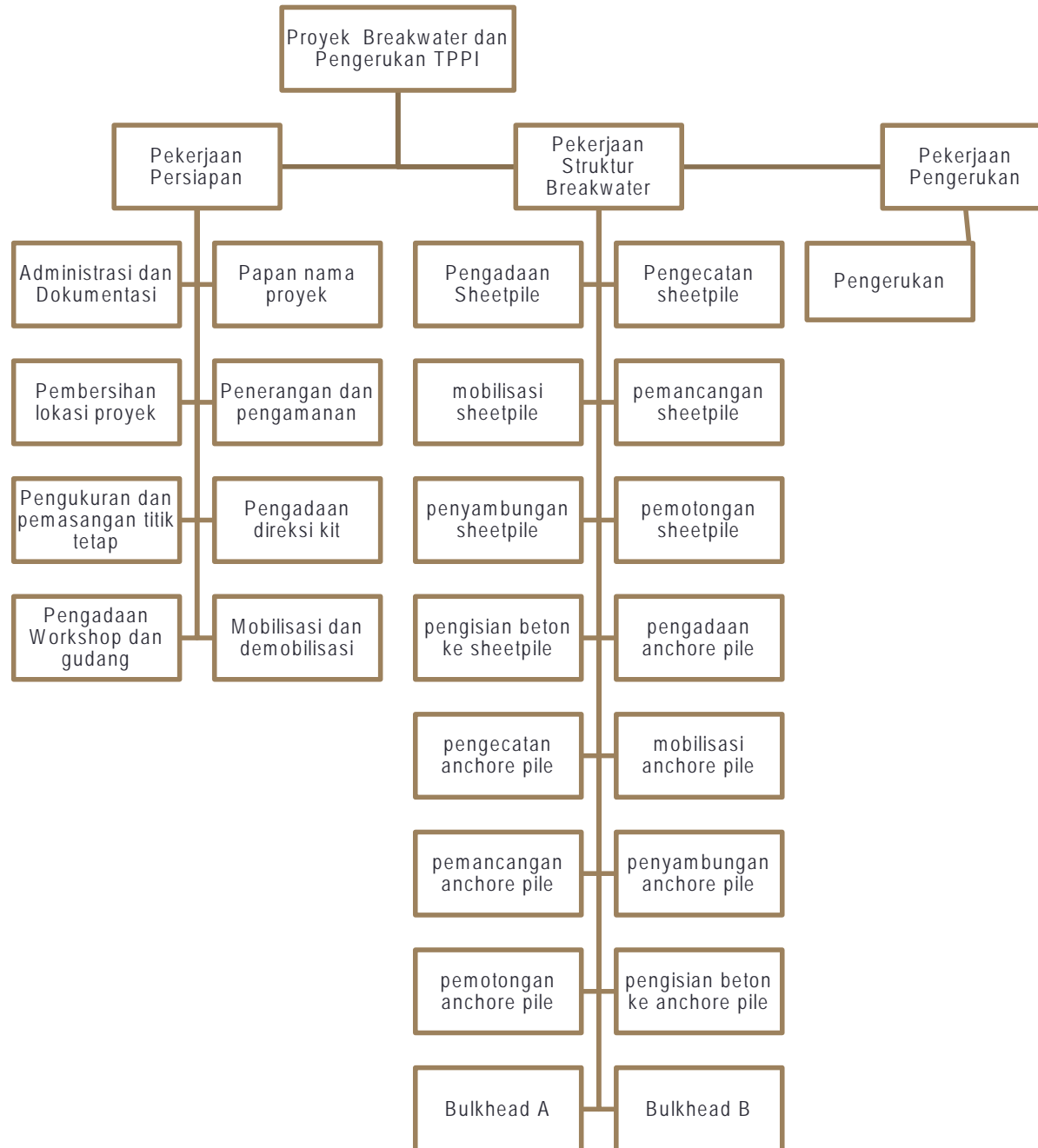
Pengerukan





WORK BREAKDOWN STRUCTURE DAN RAB

WBS



Rencana Anggaran Biaya

No.	Pekerjaan	Harga Pekerjaan
1	Pekerjaan Persiapan	Rp 484,877,642.17
2	Pekerjaan Struktur Breakwater	Rp 314,726,639,356.89
3	Pekerjaan Pengerukan	Rp 65,475,000,000.00
Total		Rp 380,686,516,999.06
Profit + O.H 10 %		Rp 38,068,651,699.91
Jumlah		Rp 418,755,168,698.96
PPn 10%		Rp 41,875,516,869.90
Jumlah Akhir		Rp 460,630,685,568.86
Jumlah Akhir (Dibulatkan)		Rp 460,630,686,000.00



KURVA S

Kurva S

No	URAIAN PEKERJAAN	Cost	Total Bobot	September Week				Oktober Week				November Week				Desember Week				Januari Week				Februari Week				Maret Week				April Week					
				1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4						
I	PERSIAPAN	Administrasi dan dokumentasi	Rp 107.561.600	0.0281	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351	0.000351				
		Papan Nama Proyek	Rp 869.798	0.0002	0.0002																																
		Pembersihan Lokasi Proyek	Rp 101.825.891	0.0266			0.006654	0.0066537	0.006654	0.006654																											
		Penerangan dan Pengamanan	Rp 143.970.354	0.0376			0.0004887	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489	0.000489				
		Pengukuran dan pemasangan titik tetap	Rp 5.000.000	0.0013						0.0013																											
		pengadaan direksi kit	Rp 25.200.000	0.0066						0.0033	0.0033																										
		pengadaan workshop dan gudang	Rp 25.200.000	0.0066						0.0033	0.0033																										
		mobisasi dan demobilisasi	Rp 75.250.000	0.0197		0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249	0.000249				
		Pengadaan Sheet Pile (Tiang Pancang)	Rp 185.172.480.000	48.3994							1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986	1.209986			
		PengecatanTiang Pancang	Rp 3.254.477.388.52	0.8506							0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266	0.021266				
Pembuatan Sepatu Tiang	Rp 1.517.666.067.00	0.3967							0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917	0.009917						
II	Struktur Breakwater	Mobilisasi Sheet pile (Tiang Pancang)	Rp 15.035.844.866.59	3.9300						0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875	0.081875					
		Pemancangan Sheet pile	Rp 15.714.318.173.56	4.1073						0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569	0.085569				
		Penyambungan Sheet pile	Rp 1.117.777.327.42	0.2922						0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087	0.006087				
		Pemotongan Sheet pile	Rp 435.217.902.39	0.1138																																	
		Pengisian Beton ke Sheet pile	Rp 14.055.312.956.48	3.6737																																	
		Pengadaan Anchor pile	Rp 44.224.200.000.00	11.5591							6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373	6.08373					
		Pengecatan Anchor pile (Tiang Pancang)	Rp 832.354.003.15	0.2176							0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145	0.01145				
		Pembuatan Sepatu Tiang	Rp 388.153.081.28	0.1015							0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636	0.005636				
		Mobilisasi Anchor pile	Rp 3.845.516.244.66	1.0051							0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863	0.047863				
		Pemancangan Anchor pile (Tiang Pancang miring)	Rp 3.851.580.245.87	1.0067							0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938	0.047938			
Penyambungan Anchor pile	Rp 285.878.905.29	0.0747							0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558	0.003558					
III	Pengerukan	Pemotongan Anchor pile	Rp 111.309.841.81	0.0291																																	
		Pengisian Beton ke Anchor pile	Rp 3.994.154.175.32	1.0440																																	
		Bulkhead A	Rp 16.861.681.405.57	4.4072																																	
		Bulkhead B	Rp 5.934.535.920.25	1.5511																																	
		Pengerukan	Rp 65.475.000.000	17.1135							0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676	0.855676				
		Total	Rp 382.592.336.147	100.000																																	
		WEEK				1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32
		BOBOT RENCANA				0.000351	0.000828	0.007254	0.007428	0.007743	0.007743	0.008083	0.861352	2.675124	2.723393	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	2.996284	
		KUMULATIF BOBOT RENCANA				0.000351	0.001179	0.008433	0.016176	0.023919	0.031662	0.040644	0.901996	3.579119	6.302513	9.298796	12.29508	15.29136	18.28765	21.28393	24.28021	27.2765	30.27278	33.26906	36.26535	39.26163	42.25791	45.2542	48.25048	51.24677	54.24305	57.23933	58.76593	60.28705	61.80819	63.32931	64.75105





TERIMA KASIH